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Criticality in the planform behaviour of the Ganges River meanders

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1 Criticality in the planform behavior of the Ganges River
2 meanders

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9 **ABSTRACT**

10 The critical point of planform transition from straight to meandering in the
11 wandering Ganges River is identifiable. Recent remote-sensing data indicate that four
12 similar meanders cutoff, or attempted to cutoff, after ~31–35 years, primarily due to
13 channel-aggradation. As main-channels aggrade, sinuosity is maximized for broad
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19 chaotic state. Planform behavior is described by the Jerolmack-Mohrig mobility
20 number and the Parker stability criterion, which well-define meander behavior as they
21 approach criticality and then relax via partial or completed avulsions. The results have
22 significance for river engineering, river network and stratigraphic modeling. Such an
23 approach could be of practical value when predicting the behaviors of other major
24 wandering rivers.

25 **INTRODUCTION**

26 Stølum (1996) showed that channel sinuosity oscillates across a predictable
27 critical state mediated by local cutoff (avulsion) processes. Such an adjustment is a
28 form of self-organized criticality (SOC; Bak, 1996); when the critical state is reached,
29 meanders adjust to regain order before evolving further. Using the criticality concept,
30 we show that the course of the wandering Ganges River, India (study area:
31 24.459317°N; 88.103924°E; Fig. 1) oscillates in space and time from a more ordered
32 to a more chaotic state (Stølum, 1996), without change in the magnitude and
33 frequency of external forcing. However, the SOC environment and time-scale can be
34 subject to local fixed controls (here bedrock pinch-points) that condition SOC
35 behavior (Camazine et al., 2001). The low-sinuosity river (ordered state) increases its
36 sinuosity (chaotic state) until local bank instabilities, manifest as avulsions, lead to
37 channel shortening to reach a low sinuosity value again. Meander regrowth follows.
38 Thus, the critical state is defined as the planform pattern transition point.
39 Between Farrakka barrage (West Bengal, India) and Hardinge bridge (Sara,
40 Bangladesh) three meanders occur with a further meander immediately upstream of
41 the barrage (Fig. 1). At any river kilometer, there is a low-gradient sandy main
42 meandering channel or up to three additional lesser cutoff channels. Such rivers are
43 termed ‘wandering’ (Church, 1983). Floodplains and bars have no significant
44 vegetation control. Today, the upstream bend basal control point is the Farrakka
45 barrage and at each of the other bends translation is limited by geological pinch-points
46 (Hossain et al., 2013) that impose important control on meander evolution. Eleven
47 maps (A.D. 1780–1967) reveal a persistent pattern of four meanders increasing in
48 amplitude without downstream translation until cutoffs occur over decadal time scales
49 that lead to periodic reduction in main channel length and sinuosity. In addition, 38 yr

50 of remote sensing data (Landsat Multispectral Scanner, Thematic Mapper, Indian
51 Remote Sensing Satellites Linear Imaging Self-Scanning [LISS] I and LISS III) (from
52 1972) were used to explore channel planform changes by identifying completed
53 avulsions or partial avulsions (Fig. 1). Main-channel widths and radii of curvature at
54 meander apices were quantified for each of the four meanders through time.

55 **SETTING**

56 The annual peak flow on the Ganges River usually occurs within a 1.5 m stage
57 range. Bankfull discharge is exceeded yearly, then the low natural levées are
58 overtopped by shallow floodplain flow or are breached by small cutoffs that transect
59 the major meander loops. These cutoffs scour the floodplains (Coleman, 1969) but the
60 main channel does not realign. Rather, it takes several years for the main flow to
61 adopt any enlarging cutoff channel (Fig. 1). Upstream of the Farakka Barrage the
62 sediment load is $729 \times 10^6 \text{ t yr}^{-1}$ (Wasson, 2003) which, due to the barrage, reduces
63 downstream to $300\text{--}500 \times 10^6 \text{ t yr}^{-1}$ at Hardinge Bridge (Hossain et al., 2013). The
64 barrage (constructed 1975) was fully aggraded by 1995 (Fig. 2) and much sediment
65 now passes by canal to the Bhagirathi-Hooghly River. Thus, the sediment load
66 downstream of the barrage reduces by $\sim 41\text{--}68\%$.

67 Four similar meander bends were studied (Fig. 1): one upstream (R1) and
68 three downstream (R2–R4) of the barrage. All bends developed simultaneously and
69 cutoff, or attempted to cutoff, by chute development over similar time scales (31–35
70 yr). Thus, although the remote sensing time series is too short to develop a statistical
71 assessment of cutoff frequency, there are four replicates of the cutoff phenomenon.

72 **CONDITION FOR AVULSION**

73 The avulsion condition largely is due to channel aggradation (Jerolmack and
74 Mohrig, 2007) that forces overbank flows to occur more frequently. However,

75 tightening bends deepen on their outer banks (Seminara, 2006), increasing bend flow
76 resistance causes both elevation in the outer bank flow level and increased bank
77 erosion, increasing channel width (Germanoski and Schumm, 1993). These conditions
78 jointly are conducive to avulsion. Thus, the critical cutoff condition can be determined
79 for each bend and depends on (1) channel geometry, (2) discharge, and (3)
80 aggradation rate.

81 **Channel Geometry**

82 The radius of curvature (r) was determined for each of the main channel
83 bends. The radii of curvature decreased through time, whereas the channel widths (B)
84 often increased (Hossain et al., 2013). The inability of pointbar progradation to match
85 the rate of bend apex recession, such that B increases as bends tighten, has been noted
86 elsewhere (Kasvi et al., 2015). The condition preceding a completed (or attempted)
87 cutoff and a sudden decrease in sinuosity (S) occurred when the bend radius fell to
88 between 5000 m and 2000 m. Thus, cutoff likelihood, in part, can be defined by the
89 ratio r/B (Howard and Knutson, 1984). To cut off, the river must flow overbank and
90 avulse by rapid erosion of the levée and floodplain surface. The minimum condition
91 for overbank flow is bankfull discharge (van Dijk et al., 2014) plus super-elevated
92 outer bank flow. For bankfull flow ($Q_b \sim 56,633 \text{ m}^3 \text{ s}^{-1}$; Coleman, 1969), for the
93 channel width ($\sim 4000 \text{ m}$) immediately before cutoff occurs, and for the minimum
94 radius of curvature (2000 m), the water surface super-elevation (Δy) is:

$$95 \quad \Delta y = \frac{c\bar{U}^2\bar{B}}{rg}, \quad (1)$$

96 where c is a coefficient (0.5) for subcritical flows, the bankfull bulk-flow velocity
97 $\bar{U} = Q_b/\bar{h}\bar{B}$, where \bar{B} and \bar{h} are average values of the channel width and depth (h)
98 at bankfull, and g is gravity. Bankfull velocity is low (on the order of 1 m s^{-1}) such
99 that inertia is small. Thus, super-elevation at the bankline is no more than $\sim 50 \text{ mm}$

100 above the channel center water surface. So, for these shallow overbank conditions,
101 near-bankfull flows alone are not likely to induce cutoff (Howard, 2009). Rather,
102 sustained outer-bank erosion, causing r/B to continue to decrease and further channel
103 aggradation, is required to elevate water levels additionally. Alternatively, discharges
104 much above bankfull are required.

105 **Discharge**

106 Rapid erosion of the outside bend will occur if discharge is adequate to entrain
107 bank material for a sufficient time (Edmonds et al., 2009). Bendway flow resistance
108 will reach a maximum as the radius of curvature reaches a minimum value. The
109 straight channel shear stress (τ_T) due to skin friction (f) is:

$$110 \quad \tau_T = \rho g R S_e = \rho f \bar{U}^2, \quad (2)$$

111 where ρ is the density of water, R is the hydraulic radius, and S_e is the energy slope.

112 The hydraulic radius is ~16 m with a regional bankfull energy slope: $5-6 \times 10^{-5}$
113 (Coleman, 1969). These data provide an estimate of unit shear stress on the order of
114 10 N m^{-2} . Determining additional form resistance induced by bends is complex (e.g.,
115 Chang, 1983). However, for illustrative purposes, we utilize the method of Leopold et
116 al. (1960) to estimate bend form shear stress ($\tau_B = \rho g \bar{h} S_\zeta$) using an energy
117 dissipation term ($\bar{h} S_\zeta$):

$$118 \quad \bar{h} S_\zeta = \frac{\bar{U}^2}{g} \left(\frac{B}{r} - 0.5 \right) - h(1 + 1.5F^{0.66}), \quad (3)$$

119 where F is the near-bank Froude number for given local depth h . For the minimum
120 values of r/B , the form-induced shear stress can be up to an order of magnitude larger
121 than the skin shear stress. For greater r/B values, the form resistance declines. When
122 avulsions were imminent, values of r/B are consistent for all four reaches ($\overline{1.29}$;
123 standard deviation 0.72; $n = 27$) but smaller than those values (~ 3) reported by Begin

124 (1986) and Howard and Knutson (1984) for the condition when bank retreat through
125 erosion is maximized. Thus, the ability of the channel to develop significant form
126 resistance and adjust through increasing sinuosity is maximized for small radii of
127 curvature and decreases for bends of greater amplitude. However, increasing form
128 resistance as bends tighten induces a backwater effect and super-elevation that is
129 conducive to cutoff before r/B is maximized, preventing further bend tightening and
130 relaxing the system by reducing sinuosity.

131 **Aggradation**

132 The aggradation rates for meander bends R2–R4 are unknown, but for R1,
133 channel aggradation and subsequent attempted avulsion were induced by backwater
134 sedimentation above the barrage. A linear and then asymptotic approach to constant
135 zero aggradation is typical of impoundments (Wu et al., 2012) and provides a
136 maximum aggradation rate, $\sim 0.18 \text{ m yr}^{-1}$, to use as a scalar in reach 1 (Fig. 2A). Bend
137 extension increases rapidly once $1/3$ of the impoundment depth is filled (Fig. 2B). For
138 R2–R4, the aggradation rate (V_a) is assumed proportional to the reduction in the
139 sediment load ($V_a = 300/729 \times 0.18 \text{ m yr}^{-1}$) below the Farraka barrage. As the system
140 aggraded, channel sinuosity increased and attempted avulsions and cutoffs developed
141 (Figs. 2 and 3). As channel aggradation rate, T_A , mediates the rate of lateral erosion,
142 T_C , the latter a key variable to define critical state (Stølum, 1998), consideration of
143 $T_A:T_C$ can define the critical state of the planform pattern transition if other factors are
144 significantly subordinate.

145 **PLANFORM SCALING MODEL**

146 The model used to show the meander behavior is the Jerolmack and Mohrig
147 (2007) approach to calculate the avulsion frequency (f_A) of a river. The avulsion
148 frequency

149
$$f_A = \frac{V_a N}{\bar{h}}, \quad (4)$$

150 is known approximately. Each reach avulsed, or tried to avulse, at a time scale ~31–
151 35 yr, so f_A can be set to 0.03 for active channels $N = 1-4$, with an average channel
152 depth of $\bar{h} = 22$ m. Jerolmack and Mohrig (2007) developed a channel mobility
153 number (M) to discriminate single channel versus multichannel form:

154
$$M = \frac{T_A}{T_C} = \frac{\bar{h}}{B} \frac{V_c}{V_a}. \quad (5)$$

155 T_C is the time to migrate one channel width and V_c is the bank erosion rate. $M = T_A/T_C$
156 = 1 defines the critical planform pattern transition (Jerolmack and Mohrig, 2007). The
157 general trend of M in Figure 3 shows the temporal trajectories of reach behavior. For
158 $M \gg 1$ a single, laterally mobile sinuous channel is expected. For $M \approx 1$, then
159 transition is expected between a single channel and multiple channels. For $M \ll 1$, a
160 multichannel avulsive system is expected. In accord with SOC, few, small avulsions
161 release energy which suppresses the likelihood of large avulsions whereas large
162 avulsions increase the energy capacity of the network, which is a destabilization
163 (Stølum, 1998). Accordingly, the network is attracted to $M \approx 1$. Such a simple model
164 uses few parameters to elucidate emergent behavior without appeal to detailed
165 process.

166 M is used here with the Parker (1976) channel stability criterion (ε):

167
$$\varepsilon = S_e \sqrt{g \bar{h} B^4} / Q, \quad (6)$$

168 to define system trend through channel pattern phase space (Fig. 4), where Q is a
169 formative discharge (bankfull value). A single-thread channel should dominate when
170 $\varepsilon \ll 1$, while a braided form should be common for $\varepsilon \geq 1$. Jerolmack and Mohrig
171 (2007) argued that a plot of M v. ε discriminated between planforms representing
172 rivers *at a single point in time* across spatial scales. In contrast, we use the M - ε phase

173 space to explore meander bend evolutions *through time* as the channel morphology
174 varies across the point of criticality due to hydraulic and morphological forcing. It is
175 evident that meander R1 differs in its behavior in contrast to R2–R4, in that the Parker
176 criterion for R1 lies between values of 0.6 and 1.5 while the other meanders exhibit
177 values usually less than 0.4. The values of $M = 1$ and $\varepsilon < 0.4$ define four quadrant
178 phase spaces for channel planform discrimination (Fig. 4).

179 **DISCUSSION**

180 A power-law avulsion distribution may characterize SOC behavior but, as
181 with many studies (Hooke, 2007), our reach-length is inadequate for this test. In
182 addition, a time constant is imposed on the Ganges' SOC cutoff behavior by spatial
183 pinch points, such that cycling occurs, similar to other guided SOC phenomena
184 (Prokopenko et al., 2014).

185 So, we focused on the critical state: defining avulsion as an autogenic response
186 of a channel when it cannot adjust further through gradual variation of sinuosity
187 (Stølum 1996). As M approaches 1, there is an increased propensity for channel
188 alignment to reset by cutoff to regain low sinuosity.

189 In a flume, lacking bank stabilizing vegetation, cutoffs occurred at a small
190 value of $S \approx 1.2$, preventing the development of more sinuous channels (Braudrick et
191 al., 2009). The Ganges River also is vegetation-free and tends to avulse when S is
192 ~ 1.3 (Fig. 3). However, the situation is not simple, as a new avulsion relaxes the
193 system such that both cutoff and main channel can be simultaneously active. There is
194 not usually a simple abandonment of the main channel in favor of the new channel
195 (Fig. 1). These 'soft-avulsion' (Edmonds et al., 2011) divert some discharge and
196 sediment from the main channel (Coleman, 1969) but much load continues down the
197 main channel. The effects of cutoffs on main channel response are known poorly

198 (Seminara, 2006). However, as main channel discharge declines, deposition will occur
199 in the main channel below the avulsion point reducing channel width (Sorrells and
200 Royall, 2014); the main thalweg depth is less affected as long as the main-channel
201 discharge remains greater than the cutoff discharge. The relaxation in the system, due
202 to the soft avulsion, results in the main meander r/B increasing as B adjusts more
203 readily than r ; which sustains potential for bank erosion downstream of the avulsion
204 as flow is increasingly confined by channel narrowing through time (Coleman, 1969).
205 Thus, soft avulsion may assist a channel maintain its meandering habit and so delay a
206 catastrophic reduction in sinuosity. Notwithstanding the relaxation due to B , r also
207 increased in three of the meanders preventing or delaying avulsion (Fig. 3).

208 Meander R1, influenced by Farraka barrage backwater, cycles from
209 anastomosed-braided to a single-channel braided pattern (Fig. 4). This pattern differs
210 from R2–R4, which cycle from avulsive-anastomosed to a sinuous single-channel
211 pattern, as is typical of wandering rivers. Thus, the imposition of the barrage, with
212 consequent accelerated upstream aggradation, reductions in slope and channel depth,
213 but broadening of the channel caused a shift from a wandering to a braided pattern, as
214 indexed by the values of ϵ . Thus, our analysis indicates that rapid aggradation in a
215 wandering river (R1) leads to braiding (*vis* Carson, 1984; his wandering type II).
216 Moreover, the wandering planform is sustainable through time, with three meanders
217 (R2–R4) adjusting similarly through time from meandering to a straighter main-
218 channel planform by the development of bend cutoffs. So, the wandering habit is not
219 necessarily indicative of a channel in short-term transition between single-channel
220 meandering and braiding (Carson, 1984). To date, the reduction in sediment load
221 downstream of the barrage has not changed the channel pattern, but a more stable
222 meandering habit is predicted by Equation 5 (*vis* Carson, 1984; his wandering type I)

223 and is observed recently (Hossain et al. 2013). Consequently, a considerable time lag
224 can be associated with any transition. The similar trend in behavior of all four
225 meanders through similar time scales is highly significant in that criticality develops
226 naturally in the meandering system.

227 Clearly, the meanders are affected by the barrage. Nevertheless, the boundary
228 conditions of a critical bend radius relative to channel apex width, the imposed
229 discharge and the aggradation rate drive the development of cutoffs as indexed by M ,
230 which reduces toward unity as the likelihood of cutoff becomes pronounced. This
231 behavior develops independently of the presence of negligible bank-side vegetation.
232 Thus, although vegetation can constrain planform, its presence is not a prerequisite to
233 enable the wandering river planform to persist. By corollary, the behavior of other
234 wandering rivers could be assessed in terms of cutoff criticality. Although channel
235 behavior is explained by SOC, limitations remain; the detailed cutoff processes and
236 how changes are transmitted beyond the cutoff locale require identification.

237 **CONCLUSIONS**

238 Low-sinuosity meanders on the Ganges River behaved similarly, extending
239 over ~35 yr without downstream translation as sinuosity increased. Two meanders
240 avulsed toward the end of the period, a third developed a soft avulsion and the fourth
241 was close to avulsion.

242 The critical bend radius:width ratio of $\overline{1.29}$ was associated with avulsion. The
243 role of super-elevation was accounted in the avulsion process, but was small. Rather,
244 as shown for a barrage-effected meander, sinuosity increased once the backwater
245 developed fully and aggradation drove the avulsion process.

246 Self-organized criticality, with a mobility number (M) tracking meander
247 development, showed the critical transitional is defined by $M \approx 1$ when avulsion was

248 imminent (Fig. 4). Channel phase space (Fig. 4) defined by Parker's braiding criterion
249 and M demonstrates that the meander upstream of the barrage adjusted from an
250 anastomosed braided system to a single-thread braided channel. Downstream, the
251 system follows a wandering river trajectory varying through time from a meandering
252 to an avulsive-anastomosed planform and then returns to meandering after ~35 yr.

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335 **FIGURE CAPTIONS**

336 Figure 1. Ganges River meanders R1–R4 development in A.D. 1972–2011. Location
337 diagram.

338 Figure 2. (A) Derivation of maximum channel aggradation rate. Triangles show years
339 (Y) of aggradation; squares are years after the barrage was full. (B) ‘Full’ channel
340 aggradation accelerates meander sinuosity.

341

342 Figure 3. Mobility number and sinuosity v. year for Ganges River meanders. Circles
343 are mobility number (M) fitted with polynomial functions. Squares are sinuosity of

344 main channel; triangles are cutoff sinuosity. Black arrows are cutoff initiation dates;

345 white arrow is date of cutoff failure (see Fig. 1).

346

347 Figure 4. Channel pattern phase space: AB—anastomosed-braided; BS—braided-

348 single; AW—wandering; S—sinuous-single. Time trends shown for Ganges River

349 meanders R1 and R4.





