

Current and future role of instrumentation and monitoring in the performance of transport infrastructure slopes

Smethurst, J. A., Smith, A., Uhlemann, S., Wooff, C., Chambers, J., Hughes, P., Lenart, S., Saroglou H, Springman, S., Löfroth, H., Hughes, D.

Joel Andrew Smethurst*
Lecturer in Geotechnical Engineering
Faculty of Engineering and the Environment
University of Southampton
Southampton SO17 1BJ
United Kingdom
E-mail: jas@soton.ac.uk
Tel: 02380 598454

Paul Hughes
Lecturer in Geotechnical Engineering
School of Engineering and Computer Sciences
Durham University
Durham
United Kingdom
E-mail: paul.hughes2@durham.ac.uk
Tel: 0191 33 42450

Alister Smith
EPSRC Doctoral Prize Research Fellow
School of Civil and Building Engineering
Loughborough University
Loughborough LE11 3TU
United Kingdom
E-mail: A.Smith10@lboro.ac.uk
Tel: 01509 565179

Stanislav Lenart
Slovenian National Building and Civil Engineering
Institute (ZAG)
Dimičeva ulica 12
Si-1000 Ljubljana
Slovenia
E-mail: stanislav.lenart@zag.si
Tel. +38 612804261

Sebastian Uhlemann
Research Geophysicist
British Geological Survey
Environmental Science Centre
Keyworth, Nottingham NG12 5GG
United Kingdom
E-mail: suhl@bgs.ac.uk

Haris Saroglou
School of Civil Engineering
National Technical University of Athens
9, Iroon Polytechniou Str.
157 80 Zografou
Athens, Greece
E-mail: saroglou@central.ntua.gr
Tel: +30 210 7722440

Chris Wooff
Asset Engineer (Geotechnics)
London North Eastern (LNE)
Network Rail
Floor 3, George Stephenson House
Toft Green, York, YO1 6HP
E-mail: chris.wooff@networkrail.co.uk
Tel: 01904 389784

Sarah Marcella Springman
University Rector and Professor for Geotechnical
Engineering
Rämistrasse 101
ETH Zurich
8092 Zurich
Switzerland
E-mail: sarah.springman@sl.ethz.ch
Tel: +41 44 633 38 05

Jonathan Chambers
Team Leader Geophysical Tomography
British Geological Survey
Environmental Science Centre
Keyworth, Nottingham NG12 5GG
United Kingdom
E-mail: J.Chambers@bgs.ac.uk
Tel: 0115 936 3428

Hjördis Löfroth
Swedish Geotechnical Institute
SE-581 93 Linköping
Sweden
E-mail: hjordis.lofroth@swedgeo.se
Tel. +46 13201854

David Hughes
Senior Lecturer in Civil Engineering
School of Planning, Architecture and Civil
Engineering
Queens University Belfast
David Keir Building, 39 Stranmillis Road
Belfast BT9 5AG
United Kingdom
E-mail: D.Hughes@qub.ac.uk
Tel: 028 9097 4014

*Corresponding author

Figures: 2

Tables: 3

Words in text: ~6890

Words in tables: ~3370

This is the pre-print copy of the manuscript accepted for publication by QJEGH on 04 May 2017

Current and future role of instrumentation and monitoring in the performance of transport infrastructure slopes

Smethurst, J. A., Smith, A., Uhlemann, S., Wooff, C., Chambers, J., Hughes, P., Lenart, S., Saroglou H, Springman, S., Löfroth, H., Hughes, D.

Abstract

Instrumentation is often used to monitor the performance of engineered infrastructure slopes. This paper looks at the current role of instrumentation and monitoring, including the reasons for monitoring infrastructure slopes, the instrumentation typically installed and parameters measured. The paper then investigates recent developments in technology and considers how these may change the way that monitoring is used in the future, and tries to summarise the barriers and challenges to greater use of instrumentation in slope engineering. The challenges relate to economics of instrumentation within a wider risk management system, a better understanding of the way in which slopes perform and/or loose performance, and the complexities of managing and making decisions from greater quantities of data.

Notation

AGS-M	Association of Geotechnical and geoenvironmental Specialists Monitoring standard
CRI	Capacitive Resistivity Imaging
DEM	Digital Elevation Model
ERT	Electrical Resistivity Tomography
GPR	Ground Penetrating Radar
LiDAR	Light Distance and Ranging
MEMS	Micro Electrical Mechanical System
Radar	Radio Detection and Ranging

1. Introduction

Linear earthwork assets in the form of cuttings and embankments are a major component of modern transport systems, and their performance is critical to ensuring transport operations are safe and reliable. Earthwork slope failures pose significant hazard: failures in embankments may undermine roads and railways, slips in cuttings may cause material to obstruct transport routes posing risks to drivers and derailment of trains (e.g. Table 1), and there are numerous locations where road and rail routes span large, and often slow moving, landslides. Across Europe, field monitoring is widely used to help understand mechanisms of movement and deterioration, assess condition and risk, and provide design parameters for repair of slopes.

Geotechnical monitoring is usually only applied to earthworks or natural slopes that are causing or showing specific problems, often in the form of excessive displacements. A common approach is to drill boreholes and install instrumentation to measure soil displacement and groundwater levels:

these may be used in assessment of potential risk or early warning (if movements accelerate), or in analysis of stability or design of remedial measures. Accessing steeply sloping ground to drill boreholes for instrumentation can be costly, and monitoring of this type can only be applied to slopes causing significant hazard.

Regular assessment can identify slopes that may be at risk of failure: this is often carried out by visual inspection (looking for signs of movement), combined with information on the slope angle, and the nature of ground and potential ground water conditions. There are limitations to such assessments: vegetation can often obscure signs of ground movement; slopes may not always show signs of distress and instead fail in a brittle and rapid manner; the exact nature of ground and water conditions is often estimated. Visual inspections may have limited usefulness in predicting the onset of instability, as they provide little or no information on subsurface processes that are a precursor to slope failure. Slopes that are not necessarily known to be a hazard can fail unexpectedly, presenting problems for the safe operation of transport systems. As a result, there is growing interest from asset owners in more pervasive approaches that would enable more widespread condition monitoring of geotechnical assets. Such approaches rarely involve drilling boreholes as this would be too costly to apply to long lengths of asset; instead many apply monitoring of surface displacements or strain, soil water content, or climate. A network of sensors can also be linked by wireless connections, with data uploaded to the internet. Geophysical monitoring (e.g. by means of ERT (Electrical Resistivity Tomography) and seismic methods), remote sensing using satellites, or ground based radar or LiDAR (Light Distance And Ranging), all provide alternative pervasive approaches. However, many such systems are relatively untried for monitoring of engineered slopes, and it is not completely clear how monitored parameters such as surface displacements or soil moisture content should be used in indication of increased risk or incipient failure; there is often insufficient knowledge about slope processes to link physical parameters with risk of failure.

Where problem slopes are large, or are in very challenging terrain, continual monitoring and assessment may be applied instead of remedial measures, which may simply be impractical due to excessive size or cost. Monitoring can be used to gauge the likelihood of incipient failure, and provide early warning. In such circumstances, monitoring needs to be continuous, reliable, reported in near 'real time', with clear criteria to suit the level of expertise needed to make a judgement (Stahli *et al.*, 2014). Asset owners commonly differentiate their monitoring systems depending on function, so that a safety critical system would be defined as an "alarm" system and would have additional stipulations on its set up and use compared with a conventional "monitoring" system. For large time-series data sets, for which manual interrogation is impractical, automated systems may process and analyse data to determine when critical predefined thresholds have been exceeded (e.g. Smith *et al.*, 2014b). The reliability of an instrumentation system is dependent on continued operation of instruments often placed in challenging environmental conditions, and the setting of suitable thresholds. False alarms can be costly in terms of money, confidence and reputation if they unnecessarily halt rail and road traffic.

Instrumentation may also be used for research, to provide long records of how slopes may progressively deteriorate with time, or how long periods of climate may influence pore pressures and movements (e.g. Smethurst *et al.*, 2012; Springman *et al.*, 2012a). This information obtained from instrumentation may be vital in understanding deterioration and modes of failure (of which there may be many); this information can feed back into improved conceptual and numerical models

that seek to identify assets that may be at risk. In some geologies and environments, deterioration mechanisms are complex, and there is considerable progress still to be made in working out how to monitor these and incorporate them in models (Dijkstra and Dixon, 2010; Springman *et al.*, 2012; Briggs *et al.*, 2016).

Climate change presents an increased risk to slopes. Research starting to investigate the impact the climate change may have on transport slopes indicates that more extreme periods of climate, coupled with aging assets, may cause a higher rate of failures. Climate changes that pose a threat to engineered slopes include more extreme rainfall events (both heavy showers and long periods of rain), drought and increased freeze-thaw cycles (Springman *et al.*, 2009; Clarke and Smethurst, 2010; Bles *et al.*, 2014). A greater use of instrumentation may help to manage the risk that climate change poses to transport systems.

There is evidence that proactive management of slopes can be much more cost effective than reactive repairs following failure (Glendinning *et al.*, 2009). Instrumentation and monitoring can form an important component of a long-term earthworks asset management strategy. Asset owners are often required by regulatory bodies to show continual improvement in asset management and safety; this has included investing in greater use of monitoring to control and manage risk. Thus the opportunities to use and develop techniques for condition monitoring are now very favourable.

In summary, there are several uses for instrumentation and monitoring in geotechnical asset management; and a plethora of challenges. This state-of-the-art review seeks to consider existing conventional approaches to instrumentation for slopes (what to monitor for a range of applications); to look at new instrumentation and technology that may seek to change monitoring approaches for slopes (with examples of several systems under development/trial); and seek to 'futuregaze' at the next set of challenges that new technology will pose, and suggest how instrumentation should be developed in the future.

2. Applications for monitoring

A number of applications for instrumentation and monitoring of infrastructure slopes have been considered in the introduction, and these will be described in further detail here. These may be summarised as: i) monitoring the condition of slopes (which may include earthworks that are subject to significant changes in loading or profile, and verifying the performance of remedial measures); ii) obtaining parameters for use in design of remedial schemes (in combination with a model); iii) early warning systems to provide alarm or indication of incipient failure; iv) monitoring slopes to manage risk at the infrastructure corridor scale; v) monitoring slopes to understand mechanisms of degradation and response to trigger events, to provide better conceptual models of slope performance; and vi) in development and testing of new instrumentation. This list may not be exhaustive, but many monitoring needs should fall within one of these categories.

All applications for monitoring should have an overarching aim of assisting asset management, which may be defined as: 'co-ordinated activities and practices through which an organisation optimally and sustainably manages its assets and asset systems, their associated performance, risks and expenditure over their life cycles for the purpose of achieving its organisational strategic plan'

(Hooper *et al.*, 2009). However, each of the applications listed above may address different parts of an asset management strategy, and thus have a differing specific aim for which the type of instrumentation, reading intervals and duration, volume and processing of data, and analysis and decision making process may all be very different (Dunnicliff, 1993). Table 2 provides further consideration of these common applications. Note that Table 2 may not cover all applications, and there are also other ways of categorising monitoring approaches and systems (e.g. see Hooper *et al.*, 2009).

Members of the COST Action have provided details for a number of key example case histories, for which extensive monitoring data sets are available, covering a range of the applications above. Some are referenced in the 'example case histories' column of Table 2, and full details of the sites, including owners of the datasets are given on the Action website www.bgs.ac.uk/cost1202/ (where they are labelled "WG2 completed proformas").

3. What to monitor

An instrumentation and monitoring scheme should be designed and set up to achieve specific aims (Dunnicliff, 1993; Chapman *et al.*, 2012): six applications with different aims have been considered in Section 2. The intended aims of the scheme should dictate the monitoring objectives, which lead to detailed design of instrumentation type, number of instruments, method of installation, data collection approach and reading interval, and how the data is stored, analysed and interpreted. The design of a monitoring scheme should be guided by previous site investigation information, and in some cases a detailed ground model (Fookes, 1997) and the predicted hazard.

This paper does not intend to be an exhaustive guide to all available types of instrumentation, however suggestions for the parameters that could be monitored for each of the applications of monitoring are given in Table 2. These are only indicative, and may vary considerably for the wide range of possible sites and geology that could fall into each category.

The commonly measured parameters are:

- *Ground displacements.* These are commonly measured using inclinometers, extensometers, tilt meters, and crack meters (measuring lateral, vertical, rotational and extensional movements respectively). There are also many approaches to measurement of surface displacement, such as using photogrammetry, radar interferometry and LiDAR. Displacement or strain tends to be fairly easy to measure, and in-ground instruments in particular can do so with considerable precision, if installed and read carefully. Measurements can show if ground displacements are taking place, to what depth movements occur, and the magnitude of displacements. It is notable that slope stability is controlled by stress (the strength of soil and rock materials, as input into a stability analysis), but stresses in the ground are difficult to measure and may be dependent on the stress history of the soil, which is often unknown. Strains (or displacements) are measured instead. However, to gain understanding of the failure mechanism from these measurements there is generally a need to understand the stiffness and deformation behaviour of the soils

concerned. Trying to judge incipient failure using displacements in very stiff (or very soft – in the case of some glaciomarine clays) brittle materials, may be difficult.

- *Ground water pressures.* Increased strain or complete failure in many slopes is caused by changes in effective stress, in turn caused by increases in pore water pressure. Thus pore water pressures are commonly monitored, using a range of differing types of piezometer. In partially saturated slopes, stability may be aided by pore water suctions, and instruments that can measure suction or loss of suctions may be important (see Ridley *et al.*, 2003; Springman *et al.*, 2012).
- *Climate or weather.* Rainfall is commonly monitored, as this has a direct influence on saturation of the ground and soil pore water pressures. Depending on the nature of the ground, periods of prolonged heavy rainfall, over hours, days or months will cause pore water pressures to rise, possibly triggering failure. Longer term records of rainfall, often combined with evaporation or evapotranspiration to give effective rainfall, can be used as an indication of increased periods of risk of slope instability (Clarke and Smethurst, 2010). Very short high intensity rainfall events can trigger slope failure, and are also often of interest. Temperature, and in colder climates, ground temperature, is also important; for example, thawing of frozen ground can lead to increased water pressures which may destabilise slopes.

There are a wide selection of monitoring approaches available for slopes, including different modes of sensor deployment (explored further in Section 4), the measurement of parameters not listed above, and use of techniques that are less well established and/or still in development. The selection of instrumentation to meet the specific objectives of a monitoring scheme usually considers the accuracy, precision, sensitivity, reliability and spatial and temporal resolution of different techniques (Dixon *et al.*, 2015). Detailed descriptions of well-established geotechnical instrumentation approaches are given by Dunncliff (1993), and are also categorised in the recent European geotechnical monitoring standard (BS EN ISO 18674-1, 2015). Novel monitoring approaches will be considered in Section 5.

Comments on the frequency of readings, and interpretation of resulting data, for the six different categories of monitoring application, are given in Table 2. Some of the applications that require large quantities of data to be analysed rapidly remain quite challenging, and some of the issues surrounding these will also be discussed in Section 5.

4. How to monitor

Monitoring can be carried out using a wide range of modes of sensor deployment - for example, from repeated manual measurements within a borehole for determining changes at a site scale, to satellite-based sensors for monitoring ground surface displacements at a regional scale. Key distinctions include: i) ground based versus remotely located sensors (airborne/satellite); ii) static versus dynamic (moving) sensors; iii) surface versus subsurface information; iv) point sensors versus spatial or volumetric monitoring technologies; v) permanently deployed sensors versus manually repeated measurements with temporary sensors; and vi) telemetric versus manual data retrieval. The mode of deployment has major implications for coverage, spatial and temporal resolution, and the cost of monitoring.

Remote sensing techniques using airborne and satellite based sensors can provide a very cost effective means of acquiring high resolution information of the ground surface over very large areas (Castagnetti *et al.*, 2013; Cigna *et al.*, 2014; Hardy *et al.*, 2012; Hugenholtz *et al.*, 2015; Miller *et al.*, 2012; Wasowski *et al.*, 2014), but are generally limited in terms of temporal resolution (which is based on satellite orbits or flight schedules) and provide only surface or very near-surface information. For smaller infrastructure slopes (versus large landslides) spatial resolution may also be insufficient, and remote sensing techniques can also be impeded by the dense vegetation cover present on some infrastructure slopes (e.g. Miller *et al.* 2007).

Dynamic ground based sensing systems, such as terrestrial LiDAR (Light Detection and Ranging; Fan *et al.*, 2014; Lato *et al.*, 2009 and 2012; Marjanovic *et al.*, 2013), radar interferometry (Springman *et al.*, 2012; Caduff *et al.*, 2014), ground penetrating radar (GPR; Donohue *et al.*, 2011 and 2013; Silvest *et al.*, 2013) and capacitive resistivity imaging (CRI; Kuras *et al.*, 2007) can obtain greater spatial and subsurface information, but are limited in terms of temporal resolution by the need for manual data collection, and therefore can be expensive when frequent (i.e. high temporal) resolution monitoring is required.

Point sensors can give very good resolution and accuracy, but are inherently limited in coverage (i.e. they measure only within the immediate vicinity of the sensor) – but spatial imaging techniques, such as electrical resistivity, seismic methods, and ground penetrating radar (Donohue *et al.*, 2011; Loke *et al.*, 2013) can complement point information and help with interpretation in ground/groundwater conditions that are heterogeneous. Wireless sensor networks (Gong *et al.*, 2013) and fibre-optic approaches (Zhu *et al.*, 2015) have been developed that can also provide information at increasing spatial scale. Permanently deployed point sensors coupled with low power electronics and data telemetry can achieve very high temporal resolution and near-real-time information delivery (Chambers *et al.*, 2014; Smethurst *et al.*, 2006). Systems that operate remotely and automatically and interface with a wide range of permanently deployed sensor types are becoming increasingly well developed (Intrieri *et al.*, 2012).

5. New Instruments and innovation

New forms of instrumentation and the increasing ability of computing and the internet to distribute, manage and process large amounts of data provide exciting opportunities, as well as challenges, for slope monitoring. This section looks at a number of developing monitoring technologies, their maturity (whether they are at early phases of development, or becoming increasingly established e.g. with numerous field trials), and the changes that they will or may provide in monitoring of infrastructure slopes for a wide range of purposes. It also considers potential changes that more sophisticated monitoring systems may have on management of data, decision making and communication.

New measurement technologies

A range of new monitoring technologies are being used or developed for monitoring of slope stability, and a number of these, with their abilities, limitations and maturity, are described in Table 3. It should be noted that Table 3 is not exhaustive, as turning to landslide monitoring gives other

novel approaches, such as using extensometers running parallel with the slope surface (Wang *et al.*, 2008). The constraints on space also mean that it is not possible to include all advantages or limitations, particularly those relating to very specific applications.

The novel forms of instrumentation in Table 3 seek to provide a range of improvements over conventional techniques, including:

- Higher resolution data, both in time and space
- Lower costs, including both the cost of the instrumentation and installation - particularly the need to drill fewer or smaller boreholes, or in the case of some remote sensing approaches, drill no holes at all. Cost can be a major driver in instrument/technique selection.
- Automated monitoring – systems that collect and transmit data, and in some cases automatically process and compare it with thresholds to provide an alarm (e.g. of increasing displacements). Automated systems also reduce the need for manual measurements and the need to put personnel in potentially hazardous environments.
- Greater life-span for instrumentation – for example, localised shear surface displacements of about 50 to 100 mm can render inclinometer casings unusable; in contrast, shear surface displacements in excess of 100's of mm have been recorded using Shape Acceleration Array (SAA) systems (Buchli *et al.*, 2013; Dasenbrock, 2014) and active waveguide AE monitoring systems (Smith *et al.*, 2014).

Several of the techniques in Table 3 are reaching maturity, and are starting to be commonly adopted for geotechnical and structural monitoring (e.g. the Shape Array), while others are still in the earlier stages of development. Some are well established monitoring techniques, but their use for infrastructure slopes has been limited (e.g. optical fibres), and they still require application specific development, with careful trials before wider application to the transport network.

Several of the relatively new techniques are being actively developed by members of COST TU1202: the British Geological Survey have been developing ERT for earthworks moisture monitoring (e.g. Chambers *et al.*, 2014; Gunn *et al.*, 2015), and Loughborough University, UK, have been developing and are now starting to commercialise an acoustic system for monitoring slope displacement rates (called ALARMS; Smith *et al.*, 2014a; Smith *et al.*, 2016). Both of these systems show considerable promise: ERT as a means of imaging moisture changes in earthworks, and ALARMS as a low cost warning system for slope movement. Both have been installed in an embankment research facility at Nafferton, Northumberland, UK, to test their abilities against conventional instrumentation (Fig. 1; for further details, see Hughes *et al.*, 2009; Glendinning *et al.*, 2014); such facilities are valuable for testing new approaches in a controlled environment.

Table 3 identifies three techniques that have been little used so far for monitoring infrastructure slopes and which all show some promise, particularly as more pervasive approaches for condition monitoring of long-lengths of asset at relatively low cost. These are:

- Optical fibres used to measure surface strain in slopes (rather than in a borehole). As the monitored fibre can be long, the technique is potentially suited to monitoring significant lengths of asset. Fibres could be buried longitudinally, a short distance below the crest of a slope. The limitations and challenges are the relatively high cost of the equipment needed to read the strain in the fibre (although this is reducing in price), the need to correct for

temperature effects, and the uncertainty as to how the fibre will deform in response to slope movements. Time domain reflectometry (TDR) does not measure strain, but can identify the location where distortion takes place within a coaxial cable, and thus may be able to perform similar role, potentially at lower cost.

- Remote sensing technologies such as LiDAR, and photogrammetry, using data from satellites, aerial vehicles or terrestrial systems. Both techniques are becoming common for terrain mapping and monitoring surface change for large landslides and rock slopes. The methods could be used to measure surface deformation of infrastructure slopes, but challenges include developing a suitable monitoring platform (rail or road vehicles, or an aerial approach), a system for handling large quantities of data (point cloud data from LiDAR; images for photogrammetry), and the resolution and accuracy of surface change detection including in the presence of vegetation.
- Wireless sensor systems, with wirelessly networked probes such as tiltmeters and moisture content probes used across/along an asset. These are already being developed for slope monitoring applications, particularly to provide alarm of slope movements (Network Rail, 2015). If a record of measurements is required for condition monitoring, transmission of large quantities of data has significant power demands, and there is still some uncertainty as to how surface or near-surface point measurements can be used to indicate deterioration or incipient failure of a slope.

All of the above require further investigation and then potentially development and testing for use with infrastructure slopes. In development of new approaches, collaboration between asset owners, instrumentation contractors and research institutions is important to ensure any new methods align to practical monitoring/asset management needs.

Datalogging and transmission

Not included explicitly in Table 3 are recent advances in datalogging and transmitting technologies, which may be summarised as follows:

- Use of less power – commercial datalogging systems can operate with low power consumption, particularly to monitor instruments and store data, such that it is possible to run small dataloggers for many months or even years from a single small battery cell. Transmission of data wirelessly has a greater power need, and batteries then need charging systems such as fuel cells or photovoltaic panels, although approaches to careful use of power, such as turning on only once every hour to transmit data, can be adopted. Energy harvesting from vibration is also used, for which a number of commercial systems are available (e.g. Perpetuum, 2016).
- Ability to transmit greater quantities of data at speed – new 3rd and 4th generations of mobile data technology means it is now possible to send significant quantities of data via mobile phone networks. Local wireless data networks that transmit between adjacent monitoring nodes are also becoming commonplace, and are particularly helpful in geographically diverse systems.
- On-site data processing – the reducing cost of computing power and bespoke circuitry mean that it is now possible to have systems that monitor and process data continuously. This has

been critical for the development of some novel systems, e.g. acoustic emission monitoring (Dixon *et al.*, 2015), and monitoring by geophones and accelerometers.

All of the above allow systems that require less human intervention, in readings, downloading data, and in maintenance (e.g. changing batteries). This is likely to reduce costs, and avoid the need to put people into remote and potentially hazardous environments.

Data management

The reducing cost of electronic in-place sensors and improved datalogging systems mean that it is now possible to both install more sensors and take and store many more readings from instruments than was possible in the past. This enables a much better granularity of spatial and time-based information; for example, readings every few minutes rather than days or even weeks apart can provide truer representations of physical processes, such as how water pressures may react to extreme short-duration rainfall events. This level of detail can be helpful in assessing risk, as well as in understanding the physical processes that take place. Such short-interval readings are essential to real-time alarm systems.

The disadvantage is more data to transmit, store and process. However there are increasingly sophisticated commercial systems that collect and store data, process it into engineering units, and post it onto secure web portals where it can be viewed. Alarms can be set to alert key decision makers if certain pre-set trigger levels are exceeded. Standardised data formats such as AGS-M, which enable easier sharing of information, are becoming common (Richards *et al.*, 2003). These are likely to become more important as assets are monitored over longer periods, giving flexibility in updating hardware and software and interoperability between proprietary systems. There have also been advances in commercialisation of techniques for processing data – such as in software for photogrammetry applications.

Collection and monitoring of more information is part of a technological trend towards ‘big data’, which is becoming increasingly important across wide areas of the European economy. Data on engineered slopes may be generated during design, construction and operational phases, i.e. the whole life cycle of the asset; geotechnical monitoring information may be a part of this data-set. Many large highway and railway infrastructure owners increasingly store information on their assets within large databases, many of which are linked to geographical information systems (GIS). These are a digital representation of the physical and functional characteristics of assets, and act as a resource for sharing and visualising information and knowledge. For example, the United Kingdom highway agencies have a system known as HAGDMS (Highways Agency Geotechnical Data Management System; Morin *et al.*, 2014), in which information is associated with relevant assets in geographical space. These systems share many similarities with Building Information Modelling (Eastman *et al.*, 1974), although there are differences, for example the linear nature of the infrastructure makes 2D rather than 3D representation of an asset more appealing.

Traditional monitoring approaches produce periodic reports, which might be attached to an asset within the GIS system. The capability of current systems to hold large data sets is less certain, and may become challenging as the number of sensors and frequency of readings increases. However, GIS systems that distribute on a fine spatial scale risk information, often in real time (for example, linked to antecedent and forecast rainfall), are becoming more commonplace, and it is plausible that

in the future this could include near real-time weather or asset monitoring data (e.g. local rainfall, or soil water content). A good example of this is the Norwegian national system XGEO (Fig. 2; www.xgeo.no).

6. Decision making and communication

Monitoring data is commonly used to make a range of decisions about infrastructure slopes, including assessing risk of failure, and the need for interventions such as stabilisation works. Where monitoring is already in place the asset will usually have already been identified as being at risk and there may be a requirement to make decisions (such as to reduce traffic speed or completely close a route) rapidly to maintain safe operations. Formal frameworks for these decisions vary according to operator (Highways Agency, 2010; IPWEA, 2006; CEDR, 2011) and are usually linked directly to risk assessment frameworks (either generic or site specific; ERA-NET, 2010). In some instances, exceedance of a particular threshold value(s) will result in automatic responses, which will then be validated by a responsible engineer. It is important that a control/decision making framework carefully sets out the responsibilities of personnel that will be involved, and that decision makers have appropriate experience and confidence to ensure good judgements.

Setting or choosing appropriate thresholds against which to assess monitoring data can be difficult, as many infrastructure slopes are unique in construction history, geometry and geological conditions. Where the ground is actively moving, rates of displacement can be monitored, but it can nonetheless be difficult to decide the risk posed by an increased rate of movement. Predicting the transition from slow acceptable movement to rapid catastrophic movement is difficult. Sometimes it is necessary to monitor slopes over a period of time to assess movements in response to hydrological changes to understand how local thresholds may be set (e.g. Reid *et al.*, 2008; Eberhardt *et al.*, 2008); this observational approach is common in managing uncertainty in geotechnical engineering (Chapman *et al.*, 2012). Thresholds levels can be set using a green/amber/red system of increasing risk with colour (e.g. the XGEO system in Fig. 2 uses this in context of national hazard mapping). Thresholds are often based on safety or performance criteria, such as the need to maintain railway track line and level.

Where monitoring systems play a critical safety role, reliability of the instrumentation and monitoring system is particularly important. False alarms can be a major issue, particularly if these result in rail and road traffic being halted unnecessarily, or are in remote sites that take an engineer a long time to reach. It is important that instrumentation systems are designed to be robust, and that may include incorporating redundancy, or providing other means by which alarms can be rapidly checked by experienced personnel such as providing video or images of the site accessed via the internet (e.g. Network Rail, 2015).

In the context of engineered slopes, important decision makers will include the earthworks engineering/asset management team, who are typically responsible for the performance and safety of assets in a particular region of the transport network, and operations personnel involved with ensuring the smooth running of transport systems. Others potentially using monitoring information to make decisions include strategic transport planners within government who will make investment decisions for major upgrade programmes or for new routes, and the general public who will make

decisions on journey planning when provided with appropriate information, e.g. enhanced risk of disruption due to extreme weather.

Forecasting and communicating periods of enhanced risk

Risk is often assessed at the corridor or network scale, where there may be an increased risk of failure and thus disruption to operations during and after long periods of heavy rainfall, or prolonged very dry periods (which may cause shrinkage of clay earthworks). There are established methodologies for assessing geotechnical risk over lengths of corridor (Gavin *et al.*, 2016) and these can incorporate antecedent conditions and/or forecast weather, combined with geological and topographical information. The Norwegian XGEO system uses hydrological (soil water content) information to assess potential risk of landslips on 1 km grid squares at a national scale (Fig. 2; Boje *et al.*, 2015; Devoli *et al.*, 2014), and a demonstrator system is being developed for the UK London to South West rail routes (Sadler *et al.*, 2015) that determines earthworks risk based on geology, soil moisture conditions and forecast rainfall. More sophisticated systems could incorporate underlying slope failure models based on approximate soil properties and the geometry of the earthworks, although it could be challenging to predict failure within individual slopes as key data (geometry, geology, condition) and models of failure are often insufficient or too simplistic (Glendinning *et al.*, 2015; Elia *et al.*, 2016). Nonetheless, such a system could be valuable if coupled with near-future weather data (e.g. impending storms) to assess the broader probability of slope failure causing disruption to transport operations. Local monitoring data could also be incorporated within a system to improve estimates of risk, although this may require processing of large amounts of data through multiple iterations of models, requiring significant computational resources.

XGEO is publically available in Norway, and is used to help communicate risk and thus the potential for travel disruption (from a range of hazards including geotechnical failure) to the general public. This information provision can be key in helping the public to make informed decisions about how and when to travel.

7. The future – where do we go next?

Many European countries have mature road and rail systems, some of which are now quite old; for example, many rail earthworks have been used for 100 years or more. Despite their age, the demand for travel is growing in many European countries: for example, rail use in the UK has grown by more than 50% since 2000 (Powrie, 2014) and is expected to double in the next 25 years. The public expectation for performance and reliability is also greater, and this poses challenges for linear infrastructure systems in which elemental failure can cause disruption to large lengths of route. Increasing safety is also expected of public infrastructure systems; in the United Kingdom during periods of adverse wet weather failure of earthworks infrastructure can pose a safety risk to the travelling public and railway staff greater than for other infrastructure types (such as track, signalling and bridges) combined (Hutchinson, 2015). Climate change may also affect asset performance. The main driver for slope failure is rainfall, and it is possible that a hotter future European climate will see rainfall arrive in more intense storm events. Drier summers may also pose difficulties for earthworks, causing cracking and shrinkage problems in clay soils (Clarke and Smethurst, 2010). Both

the public and transport operators want safe and disruption free systems, and this is likely to be a driver for change to the way that assessment and monitoring of geotechnical assets is approached.

Monitoring data is also needed to help understand and reduce failure in newly built infrastructure. New road and rail systems often operate at higher speed, and the hazard posed by running into slipped debris (causing derailment or crash) is greater. The lessons from understanding deterioration and failure in older systems is needed to help design, monitor and maintain new geotechnical assets.

This is also an exciting time for monitoring technologies. The emergence of the internet, increasingly powerful wireless transmission and data recording technologies, cheaper sensors, enhanced remote sensing technologies, the ability to process large amounts of data in real time, and greater commercialisation of monitoring technology across domains, are all making possible things not available to us even a few years ago. All of the above are feeding into new technology development in geotechnical monitoring; the above sections in this paper detail some novel approaches being developed by COST Action members, although there are also many others.

Specific slopes with known stability problems require careful monitoring using more conventional instrumentation (inclinometers, piezometers) to manage the risk that they present. However, generally the majority of earthworks will not be monitored, subject at best only to visual inspection by experienced personnel at frequencies between annual and 10 yearly. Some of these slopes do and will fail unexpectedly, causing disruption, at considerable cost to the economy. In order to try and monitor longer lengths of earthwork, operators are increasingly keen on more pervasive condition monitoring approaches (i.e. those that monitor surface displacement and soil water content etc. over long lengths of asset at low cost), that may be able to highlight earthworks which are showing initial distress. Such systems could require little human intervention; remote sensing, wireless and internet technologies may all allow systems that are significantly automated.

There is also considerable potential to enhance the way that we view, manage and disseminate monitoring data using the internet; this paper has looked at two examples in the Norwegian XGEO and UK London and South-West demonstrator systems. Condition monitoring data could be used in the future to determine earthwork risk along significant lengths of route using physically based models; this has the potential to be updated in near-real time with, for example, forecast weather to show future probabilities for earthwork failure and thus disruption to transport operations.

While such systems are very desirable, there are of course significant challenges to achieving these types of monitoring systems. These can be summarised in three points:

The assets: earthworks are difficult. They can be very variable in terms of geometry and material properties, there can be local 'defects', they are often covered with vegetation that can make assessment and condition monitoring difficult, and there are multiple modes of failure, some of which are complex and not well understood (Tang *et al.*, 2016). Generally we need a much better understanding of the condition of these assets and the way in which they perform (or fail). This is also needed for the development of more pervasive monitoring approaches – for long lengths of asset what are the indicators of loss of performance? Instrumentation and monitoring data fundamentally underpin the models of physical asset behaviour, and risk, that are being explored further in other parts of the COST Action. The collection, storage, analysis and dissemination and sharing of more and better quality monitoring data can provide the information and models to

properly understand modes of failure and deterioration, and the level at which to set thresholds for intervention. Any future automated system relying less on human input will be dependent on better models. The COST Action provides opportunities for closer collaboration and sharing of data between, for example, asset owners and research bodies.

The economics: new monitoring technologies and pervasive condition monitoring approaches offer promise, but there must be a good economic case for their use. Investment in more widespread use of monitoring needs be based on savings to the economy from fewer failed earthworks and less disruption. It is doubtful that thus far the case is made in its entirety – the technologies and understanding of earthworks required to make these monitoring approaches work are incomplete, and asset owners often don't have data on delay costs needed. This will change, as the technology and our expectations of aging infrastructure systems also change. Regulatory bodies, government and public expectation will play a role in challenging operators to show continual improvement in safety and management systems. Many of the new instrumentation approaches described above have also been developed using national government and European Union grants, with financial and other support from road and rail asset owners. Continued strong investment in the development of technology for monitoring of earthworks, and a pro-active approach to seeking to prevent failure, will be key.

Technological and human systems: the paper has described the developments in instrumentation for monitoring earthworks, with many systems providing enhancements in monitoring ability, reliability, longevity, cost, and the quality and quantity of data obtained. Several new techniques are very promising, but need further development for use in infrastructure slope monitoring. The ability to monitor more slopes at greater spatial and temporal resolution also requires handling, processing and analysis of significantly more data. This follows the economic trend for understanding systems using 'big data'. Automated systems that analyse large quantities of data are desirable, although their application may have limits – it could still best to have human judgement of the data in major decision making processes (e.g. before stopping traffic). This introduces the need to have enough suitably trained people to understand and review situations and make good/consistent decisions, and, where appropriate, the use of standardised monitoring (avoiding having large numbers of highly bespoke systems) and centralised control. The human influence in decision making requires careful processes and clear risk/decision/response plans are an essential part of major monitored systems.

These are all significant challenges, and it will require time and investment to achieve enhanced monitoring of European transport systems. These can be overcome more easily if we collaborate, and share ideas and data as European partners – something the COST Action has been trying to achieve.

8. Conclusions

- i. This paper has explored the context and background to instrumentation and monitoring of infrastructure slopes in Europe. It has considered typical applications for monitoring, spanning from systems to warn of imminent failure, to monitoring for research to better understand the physical processes that take place in slopes.

- ii. A number of novel instrumentation approaches have been described; some of these are gaining widespread use, and others are at the research and development stage. New technologies and systems are providing enhancements in monitoring ability, reliability, longevity, cost, and the quality and quantity of data obtained.
- iii. There is considerable potential for the changing demands and expectations of infrastructure systems and new monitoring technologies to completely change the way that slopes are monitored in the future. It will likely be possible to monitor greater lengths of earthwork, with the intention of providing warning of and reducing incidences of unexpected failure (i.e. condition monitoring), rather than the fairly reactive monitoring approaches commonly seen today.
- iv. Several new techniques for monitoring longer lengths of slope are promising, but need application specific development before use for infrastructure slope monitoring. These techniques include optical fibres, LiDAR and photogrammetry, and wireless sensor networks.
- v. The ability to monitor more slopes at greater spatial and temporal resolution requires handling, processing and analysis of significantly more data. Automated systems that analyse large quantities of data are desirable, although human judgements in conjunction with careful decision making frameworks will still be required.
- vi. Improved modelling of risk at the route scale, and improving database and internet systems may allow the possibility of hazard or risk maps that update continually with asset condition-monitoring data and current or forecast climate. Such systems could prove invaluable to transport operators, as well as communicating risk to the travelling public. This paper has looked at examples of such systems in use and in development.
- vii. To enable more widespread monitoring and better communication of risk, improved models of slope performance and failure are required, as well as a better financial case. Parts of this are discussed in more detail in other papers from COST Action TU1202. Both will be underpinned by improved quality, collection, analysis and communication of monitoring data from infrastructure slopes.
- viii. Greater communication and sharing of data and ideas between European nations and continued investment in monitoring technologies by European transport operators and governments is required to aid the monitoring challenges elucidated above.

Acknowledgements

This paper is an output of Working Group 2 of EU COST Action TU1202 – Impacts of climate change on engineered slopes for infrastructure. TU1202 comprises four working groups, WG1 – Slope numerical modelling, WG2 – Field experimentation and monitoring, WG3 – Soil/vegetation/climate interactions, WG4 – Slope risk assessment. Outputs from each working group have been submitted to QJEG&H and are intended to be read as a thematic set. The authors gratefully acknowledge the funding for COST Action TU1202 through the EU Horizon 2020 programme, without which these outputs would not have been possible. J Smethurst was also supported by the UK Engineering and Physical Sciences Research Council grant number EP/K027050/1.

Other papers within this thematic set:

590

591 G. Elia, F. Cotecchia, G. Pedone, J. Vaunat, P. J. Vardon, C. Pereira, S. M. Springman, M. Rouainia, J.
592 Van Esch, E. Koda, J. Josifovski, A. Nocilla, A. Askarinejad, R. Stirling, P. Helm, P. Lollino, P. Osinski.
593 Numerical modelling of slope-vegetation-atmosphere interaction: an overview. *Submitted to*
594 *Quarterly Journal of Engineering Geology and Hydrogeology*.

595 A.M. Tang, A. Askarinejad, M. Brencic, Y.J. Cui, J.J. Diez, T. Firgi, B. Gajewska, F. Gentile, G. Grossi, C.
596 Jommi, F. Kehagia, E. Koda, H.W. ter Maat, S. Lenart, S. Lourenco, M. Oliveira, P. Osinski, S.M.
597 Springman, R. Stirling, D.G. Toll, V. Van Beek, P. N. Hughes, T.A. Dijkstra. Atmosphere – vegetation –
598 soil interactions in a climate change context; changing conditions impacting on engineered transport
599 infrastructure slopes. Submitted to *Quarterly Journal of Engineering Geology and Hydrogeology*.

600 K. Gavin, K. Martinović, C. Reale, M. Bačić, F. Cotechia, T. Dijkstra, K. Flesjo, D. Hutchinson, M. S.
601 Kovačević, S. Lenart, L. Librić, R. Maftai, S. Mickovski, F. Rodriguez Lopez, H. Saroglou, V. Sheshov, I.
602 Stipanovic – Oslakovic, C. Vitone, C. Wooff. Use of Risk Assessment Frameworks for the
603 Management of Transport Infrastructure Slopes in Europe. Submitted to *Quarterly Journal of*
604 *Engineering Geology and Hydrogeology*.

605

606 **References**

607 Abbott, S., Power, C., and Mian, J. (2014). *Presentation to the Slope Engineering & Geotechnical*
608 *Asset Management Conference*, London, held on the 19 November 2014.

609 Abdoun, T., Bennett, V., Desrosiers, T. Simm, J., and Barendse, M. (2013). Asset management and
610 safety assessment of levees and earthen dams through comprehensive real-time field monitoring.
611 *Geotechnical and Geological Engineering*, 31 (3), 833-843.

612 Akca, D., Gruen, A., Askarinejad, A., and Springman S. M. (2011). Photogrammetric monitoring of an
613 artificially generated land slide. *International Conference on Geo-information for Disaster*
614 *Management (Gi4DM)*, Antalya, Turkey, 3-8 May 2011. Published on CD-ROM only

615 Askarinejad, A., Casini, F., Bischof, P., Beck, A., and Springman S. M. (2012). Rainfall induced
616 instabilities: a field experiment on a silty sand slope in northern Switzerland. *Rivista Italiana di*
617 *Geotecnica* 3/2012 (Luglio - Settembre 2012), 9-30.

618 Bemis, S. P., Micklethwaite, S., Turner, D., James, M. R., Akciz, S., Thiele, S. T., and Bangash, H. I.
619 (2014). Ground-based and UAV-based photogrammetry: a multi-scale, high-resolution mapping tool
620 for structural geology and paleoseismology. *Journal of Structural Geology* 69:163–178. doi:
621 10.1016/j.jsg.2014.10.007

622 Bles, T., Bassembinder, J., Chevreuil, M., Danielsson, P., Falemo, S. and Venmans, A. (2015) *Roadapt.*
623 *Roads for today, adapted for tomorrow. Guidelines*. CEDR Transnational Road Research Programme,
624 May 2015.

625 Boje, S., Devoli, G., Cepeda, J. and Colleuille H. (2014). Landslide thresholds at regional scale for an
626 early warning system in Norway. *Proceedings of the World Landslide Forum 3*. 2-6 June 2014, Beijing

627 Briggs, K. M., Loveridge, F. A., and Glendinning, S. (2016). Failures in transport infrastructure
628 embankments. Submitted to *Engineering Geology*, February 2016.

629 Briggs, K.M., Smethurst, J.A., Powrie, W. and O'Brien, A.S. (2013). Wet winter pore pressures in
630 railway embankments. *Proceedings of the Institution of Civil Engineers: Geotechnical Engineering*,
631 166, 5, 451-465. doi: 10.1680/geng.11.00106

632 BS EN ISO 18674-1:2015 (2015). *Geotechnical investigation and testing. Geotechnical monitoring by*
633 *field instrumentation. General rules*. British Standards Institution. pp. 36.

634 Buchli, T., Laue, J. and Springman, S. M. (2016). Amendments to interpretations of SAAF inclinometer
635 data from the Furggwanhorn Rock Glacier, Turtmann Valley, Switzerland: results from 2010 to
636 2012. *Vadose Zone Journal* 15, 4, 1-3.

637 Buchli, T., Merz, K., Zhou, X., Kinzelbach, W., Springman, S. M. (2013). Characterization and
638 monitoring of the Furggwanhorn Rock Glacier, Turtmann Valley, Switzerland: results from 2010 to
639 2012. *Vadose Zone Journal* 12, 1, 1-15.

640 Caduff, R., Schlunegger, F., Kos, A., and Wiesmann, A. (2014). A review of terrestrial radar
641 interferometry for measuring surface change in the geosciences. *Earth Surface Processes and*
642 *Landforms*. 40, 2, 208-228.

643 Casini, F., Serri, V., and Springman, S.M. (2013). Hydromechanical behaviour of a silty sand from a
644 steep slope triggered by artificial rainfall: from unsaturated to saturated conditions. *Canadian*
645 *Geotechnical Journal*, 50(1), 28-40. <http://dx.doi.org/10.1139/cgj-2012-0095>.

646 Castagnetti, C., Bertacchini, E., Corsini, A. and Capra, A. (2013). Multi-sensors integrated system for
647 landslide monitoring: critical issues in system setup and data management. *European Journal of*
648 *Remote Sensing*, 46, 104-124.

649 CEDR (2010). Adaptation to climate change – Task 16 Report 2011. Conference of European
650 Directors of Roads. Available at:
651 www.cedr.fr/home/fileadmin/user_upload/Publications/2013/T16_Climate_change.pdf. Accessed
652 21 April 2016.

653 Chambers, J. E., Gunn, D., Wilkinson, P. B., Meldrum, P. I., Haslam, E., Holyoake, S., Kirkham, M.,
654 Kuras, O., Merritt, A. and Wragg, J. (2014). 4D electrical resistivity tomography monitoring of soil
655 moisture dynamics in an operational railway embankment. *Near Surface Geophysics*, 12, 61-72

656 Chambers, J. E., Meldrum, P. I., Wilkinson, P. B., Ward, W., Jackson, C., Matthews, B., Joel, P. Kuras,
657 O., Bai, L., Uhlemann, S., and Gunn, D. (2015). Spatial monitoring of groundwater drawdown and
658 rebound associated with quarry dewatering using automated time-lapse electrical resistivity
659 tomography and distribution guided clustering. *Engineering Geology*. 193, 412-420. doi:
660 10.1016/j.enggeo.2015.05.015

661 Chambers, J. E., Wilkinson, P. B., Kuras O., Ford, J. R., Gunn, D. A., Meldrum, P. I., Pennington, C. V.
662 L., Weller, A. L., Hobbs, P. R. N. and Ogilvy, R. D. (2011). Three-dimensional geophysical anatomy of
663 an active landslide in Lias Group Mudrocks, Cleveland Basin, UK. *Geomorphology* 125, No. 4, 472–
664 484.

665 Chapman, T., Skinner, H., Brown, M. and Burland, J. (2012). *Institution of Civil Engineers Manual of*
666 *Geotechnical Engineering*. Institution of Civil Engineers: London.

Chin, A. and Olsen, M. J. (2015). Evaluation of technologies for road profile capture, analysis, and evaluation. *Journal of Surveying Engineering*, 141, Issue 1.

Cigna, F., Jordan, H., Bateson, L., McCormack, H. and Roberts, C. (2015). Natural and anthropogenic geohazards in greater London observed from geological and ERS-1/2 and ENVISAT persistent scatterers ground motion data: Results from the EC FP7-SPACE PanGeo project. *Pure and Applied Geophysics*. 172, Issue 11, 2965-2995.

Clarke, D. and Smethurst, J. A. (2010). Effects of climate change on cycles of wetting and drying in engineered clay slopes in England. *Quarterly Journal of Engineering Geology and Hydrogeology*, 43, (4), 473-486. doi:10.1144/1470-9236/08-106.

Cornforth, D. H. (2012). Advances in investigation and analysis for soil landslides: Three selected topics. *Landslides and engineered slopes: Protecting society through improved understanding*. Proceedings of the 11th International Symposium on Landslides, Banff, Canada. Vol. 1, 59–71.

Dasenbrock, D. (2014). Performance observations of MEMS ShapeAccelArray (SAA) deformation sensors. *Geotechnical Instrumentation News*. June, 23-26. Available at: www.bitech.ca/pdf/GeoTechNews/2014/GIN%203202.pdf. Accessed 23 April 2016.

Devoli, G., Kleivane, I., Sund, M., Orthe, N-K., Ekker, R., Johnsen, E., and Colleuille, H. (2015). Landslide early warning system and web tools for real-time scenarios and for distribution of warning messages in Norway. In: *Engineering Geology for Society and Territory*, Eds: G. Lollino et al. Springer International Publishing Switzerland. Vol. 2, 625-629.

Di Prinzio, M., Bittelli, M., Castellarin, A., and Pisa, P. R. (2010). Application of GPR to the monitoring of river embankments. *Journal of Applied Geophysics*. 71, 53–61. doi: 10.1016/j.jappgeo.2010.04.002

Dijkstra, T. and Dixon, N. (2010). Climate change and slope stability in the UK: challenges and approaches. *Quarterly Journal of Engineering Geology and Hydrogeology*, 43 (4), pp. 371 - 385.

Dijkstra, T., Dixon, N., Crosby, C., Frost, M., Gunn, D. Fleming, P. and Wilks, J. (2014). Forecasting infrastructure resilience to climate change. *Proceedings of the Institution of Civil Engineers: Transport*. 167, 5, 269-280.

Dixon, N., Hill, R. and Kavanagh, J. (2003). Acoustic emission monitoring of slope instability: development of an active wave guide system. *Proceeding of the Institution of Civil Engineers: Geotechnical Engineering*, 156, 2, 83-95.

Dixon, N., Spriggs, M. P., Smith, A., Meldrum, P. and Haslam, E. (2014). Quantification of reactivated landslide behaviour using acoustic emission monitoring. *Landslides*, 1-12. doi: 10.1007/s10346-014-0491-z

Dixon, N., Smith, A., Spriggs, M. P., Ridley, A., Meldrum, P. and Haslam, E. (2015). Stability monitoring of a rail slope using acoustic emission. *Proceedings of the Institution of Civil Engineers: Geotechnical Engineering*. 168, 5, 373-384.

Donohue, S., Gavin, K. and Tolooiyan, A. (2011). Geophysical and geotechnical assessment of a railway embankment failure. *Near Surface Geophysics*, 9, 33-44.

Donohue, S., Gavin, K. and Tolooiyan, A. (2013). Railway earthwork stability assessment using geophysics. *Geotechnical and Geophysical Site Characterization*. 4, Vols I and II, 1519-1525.

706 Dunnicliff, J (1993). *Geotechnical Instrumentation for monitoring field performance*. Wiley, New
707 York. pp. 577.

708 Eastman, C., Fisher, D., Lafue, G., Lividini, J., Stoker, D., and Yessios, C. (1974). *An Outline of the*
709 *Building Descripton System. Research Report No. 50*. Institute of Physical Planning, Carnegie-Mellon
710 University.

711 Eberhardt, E., Watson, A. D. and Leow, S. (2008). Improving the interpretation of slope monitoring
712 and early warning data through better understanding of complex deep-seated landslide failure
713 mechanisms. *Landslides and Engineered Slopes*. Eds: Chen et al, Taylor and Francis, London. Vol. 1,
714 pp. 39-51.

715 Elia, G., Cotecchia, F., Pedone, G., Vaunat, J., Vardon, P. J., Pereira, C., Springman, S. M., Rouainia,
716 M., Van Esch, J., Koda, E., Josifovski, J., Nocilla, A., Askarinejad, A., Stirling, R., Helm, P., Lollino, P.,
717 Osinski, P. (2016). Numerical modelling of slope-vegetation-atmosphere interaction: an overview.
718 *Submitted to Quarterly Journal of Engineering Geology and Hydrogeology*.

719 ERA-NET (2010). *Risk Management for Roads in a changing climate – a guide to the RIMAROCC*
720 *method*. Road ERA-NET.

721 Fan, L., Powrie, W., Smethurst, J. A., Atkinson, P. M. and Einstein, H. (2014). The effect of short
722 ground vegetation on terrestrial laser scans at a local scale. *ISPRS Journal of Photogrammetry and*
723 *Remote Sensing*, 95, 42-52. doi:10.1016/j.isprsjprs.2014.06.003.

724 Fookes, P. G. (1997). Geology for engineers: the geological model, prediction and performance.
725 *Quarterly Journal of Engineering Geology and Hydrogeology*. 30, 4, 293–424.

726 Gavin, K., Martinović, K., Reale, C., Bačić, M., Cotechia, F., Dijkstra, T., Flesjo, K., Hutchinson, D.,
727 Kovačević, M. S., Lenart, S., Librić, L., Maftai, R., Mickovski, S., Rodriguez Lopez, F., Saroglou, H.,
728 Sheshov, V., Stipanovic – Oslakovic, I., Vitone, C., Wooff, C. (2016). Use of Risk Assessment
729 Frameworks for the Management of Transport Infrastructure Slopes in Europe. Submitted to
730 *Quarterly Journal of Engineering Geology and Hydrogeology*.

731 Glendinning, S., Hall, J. W., Manning, L. J. (2009). Asset-management strategies for infrastructure
732 embankments. *Proceedings of the Institution of Civil Engineers - Engineering Sustainability*. 162, 2,
733 111-120.

734 Glendinning, S., Helm, P. R., Rouainia, M., Stirling, R. A., Asquith, J. D., Hughes, P. N., Toll, D. G.,
735 Clarke, D., Powrie, W., Smethurst, J. A., Hughes, D., Harley, R., Karim, R., Dixon, N., Crosby, C.,
736 Chambers, J., Dijkstra, T., Gunn, D., Briggs, K. and Muddle, D. (2015). Research-informed design,
737 management and maintenance of infrastructure slopes: development of a multi-scalar approach. *IOP*
738 *Conference Series: Earth and Environmental Science*, 26 (1), 012005

739 Glendinning, S., Hughes, P., Helm, P.R., Chambers, J., Mendes, J., Gunn, D., Wilkinson, P., and
740 Uhlemann, S. (2014). Construction, management and maintenance of embankments used for road
741 and rail infrastructure: implications of weather induced pore water pressures. *Acta Geotechnica*. 9,
742 5, 799-816.

743 Glisic, B. and Inaudi, D. (2007). *Fibre optic methods for structural health monitoring*. John Wiley,
744 Chichester, UK. pp. 276.

745 Gong, C., Zeng, G., Ge, L., Tan, C., Luo, Q., Liu, X. and Chen, M. (2013). Design of long-distance and
 746 high-accuracy rail subgrade deformation monitoring system based on Zigbee wireless network.
 747 *Applied Mechanics and Materials*, Vols. 303-306, pp. 676-684.

748 Gunn, D. A., Chambers, J. E., Uhlemann, S., Wilkinson, P. B., Meldrum, P. I., Dijkstra, T. A., Haslam, E.,
 749 Kirkham, M., Wragg, J., Holyoake, S., Hughes, P. N., Hen-Jones, R., and Glendinning, G. (2015).
 750 Moisture monitoring in clay embankments using electrical resistivity tomography. *Construction and*
 751 *Building Materials*. 92, 82-94. DOI: 10.1016/j.conbuildmat.2014.06.007

752 Hardy, A. J., Barr, S. L., Mills, J. P. and Miller, P. E. (2012). Characterising soil moisture in transport
 753 corridor environments using airborne LIDAR and CASI data. *Hydrological Processes*. 26, 1925-1936.

754 Highways Agency (2010). *A risk based framework for geotechnical asset management – Phase 2*
 755 *Report*. Issue 1, November 2010.

756 Hooper, R., Armitage, R., Gallagher, A. and Osorio, T. (2009). *Whole-life infrastructure asset*
 757 *management: good practice guide for civil infrastructure*. CIRIA Report C677. Construction Industry
 758 Research and Information Association (CIRIA): London. pp. 150.

759 Hugenholtz, C., Walker, J., Brown, O., and Myshak, S. (2015). Earthwork volumetrics with an
 760 unmanned aerial vehicle and softcopy photogrammetry. *Journal of Surveying Engineering*. 141,
 761 06014003.

762 Hughes D. A. B., Clarke G. R. T., Harley R. M. G. and Barbour S. L. (2016). The impact of hydrogeology
 763 on the instability of a road cutting through a drumlin in Northern Ireland. *Quarterly Journal of*
 764 *Engineering Geology and Hydrogeology*. 49, 92-104. doi:10.1144/qjegh2014-101

765 Hughes, P.N., Glendinning, S., Mendes, J., Parkin, G., Toll, D.G., Gallipoli, D. and Miller, P. (2009). Full-
 766 scale testing to assess climate effects on embankments. *Proceedings of the Institution of Civil*
 767 *Engineers: Engineering Sustainability*. 162, 2, 67-79.

768 Huisman, J. A., Hubbard, S. S., Redman, J. D., Annan, A. P. (2003). Measuring soil water content with
 769 ground penetrating radar a review. *Vadose Zone Journal*. 2, 476–491. doi: 10.2113/2.4.476

770 Hutchinson, D. (2015). Presentation at COST Action TU1202 workshop, Ljubljana, Solvenia, October
 771 2015.

772 Intrieri, E., Gigli, G., Mugnai, F., Fanti, R. and Casagli, N. (2012). Design and implementation of a
 773 landslide early warning system. *Engineering Geology*. 147, 124-136.

774 IPWEA (2006). *International infrastructure management manual*. Institute of Public Works
 775 Engineering Australasia.

776 Jang, H. S., Kim, C. K., Lee, J. C., Lee, Y. D. and Oh, H. W. (2008). The analysis of road side slopes using
 777 RG helicopter photogrammetric system. *Proceedings of the XXIIst ISPRS Congress, Beijing. ISPRS*
 778 *Archives*, Vol XXXVII, Part B4, 395-398.

779 Kane, W. F. and Beck, T. J. (2001). Instrumentation practice for slope monitoring. *Engineering*
 780 *Geology Practice in Northern California*. Eds: Ferriz, H and Anderson, R. California Department of
 781 Conservation, Division of Mines and Geology. pp. 658.

782 Kane, W. F., Beck, T. J. and Hughes, J. J. (2001). Applications of time domain reflectometry to
 783 landslide and slope monitoring. *Second International Symposium and Workshop on Time Domain*

784 *Reflectometry for Innovative Geotechnical Applications*. Infrastructure Technology Institute at
785 Northwestern University, Evanston, USA. pp. 305–314.

786 Kuras, O., Meldrum, P. I., Beamish, D., Ogilvy, R. D. and Lala, D. (2007). Capacitive resistivity imaging
787 with towed arrays. *Journal of Environmental and Engineering Geophysics*, 12, 267-279.

788 Lato, M., Hutchinson, J., Diederichs, M., Ball, D. and Harrap, R. (2009). Engineering monitoring of
789 rockfall hazards along transportation corridors: using mobile terrestrial LiDAR. *Natural Hazards and*
790 *Earth System Sciences*. 9, 935-946.

791 Lato, M. J., Diederichs, M. S., Hutchinson, D. J. and Harrap, R. (2012). Evaluating roadside rockmasses
792 for rockfall hazards using LiDAR data: optimizing data collection and processing protocols. *Natural*
793 *Hazards*. 60, 831-864.

794 Lehmann, P., Gambazzi, F., Suski, B., Baron, L., Askarinejad, A., Springman, S. M., Holliger, K., and Or,
795 D. (2013). Evolution of soil wetting patterns preceding a hydrologically induced landslide inferred
796 from electrical resistivity survey and point measurements of volumetric water content and pore
797 water pressure. *Water Resources Research*, 49, 12, 7992–8004. doi: 10.1002/2013WR014560.

798 Lehtonen, V. J., Meehan, C. L., Lansivaara, T. T. and Mansikkamäki, J. N. (2015). Full-scale
799 embankment failure test under simulated train loading. *Géotechnique*. 65, 12, 961-974.

800 Loke, M. H., Chambers, J. E., Rucker, D. F., Kuras, O. and Wilkinson, P. B. (2013). Recent
801 developments in the direct-current geoelectrical imaging method. *Journal of Applied Geophysics*. 95,
802 135-156.

803 Marjanovic, M., Abolmasov, B., Djuric, U., Zecevic, S. and Susic, V. (2013). Basic kinematic analysis of
804 a rock slope using terrestrial 3D laser scanning on the M-22 highroad pilot site. *Rock Mechanics for*
805 *Resources, Energy and Environment*. Eds: Kwaśniewski M. and Łydzba D. 679-683.

806 Massey, C.I., Petley, D.N. and McSaveney, M.J. (2013). Patterns of movement in reactivated
807 landslides. *Engineering Geology* 159, 1-19.

808 Miller, P. E., Mills, J.P., Barr, S.L., Birkinshaw, S.J., Hardy, A.J., Parkin, G. and Hall, S.J. (2012). A
809 remote sensing approach for landslide hazard assessment on engineered slopes. *IEEE Transactions*
810 *on Geoscience and Remote Sensing*. 50, 1048-1056.

811 Miller P. E., Mills J. P., Barr S. L., Lim M., Barber D., Parkin G., Clarke B., Glendinning S. and Hall J.
812 (2008). Terrestrial laser scanning for assessing the risk of slope instability along transport corridors.
813 *International Archives of Photogrammetry, Remote Sensing and Spatial Information Sciences*. 37, No.
814 B5, 495–500.

815 Millis, S. W., Ho, A. N. L., Chan, E. K. K., Lau, K. W. K. and Sun, H. W. (2008). Instrumentation and real
816 time monitoring of slope movement in Hong Kong. *The 12th International Conference of*
817 *International Association for Computer Methods and Advances in Geomechanics (IACMAG)*. 4563–
818 4576.

819 Morin, G., Hassall, S. and Chandler, R. (2014). Case study - the real life benefits of geotechnical
820 Building Information Modelling. In: *Information Technology in Geo-Engineering*. Eds: Toll, D. G. et al.
821 IOS Press.

822 Network Rail (2015). Climate change adaptation report 2015. Available at:
 823 www.networkrail.co.uk/publications/weather-and-climate-change-resilience. Accessed 05 June
 824 2016.

825 O'Kelly, B. C., Ward, P. N., and Raybould, M. J. (2008). Stabilisation of a progressive railway
 826 embankment slip. *Geomechanics and Geoengineering: An International Journal*. 3, 4, 231-244.

827 Perpetuum (2016). Refer to website www.perpetuum.com. Accessed 30 March 2016.

828 Petley, D. N., Mantovani, F., Bulmer, M. H. and Zannoni, A. (2005). The use of surface monitoring
 829 data for the interpretation of landslide movement patterns. *Geomorphology*. 66, 133–147. doi:
 830 10.1016/j.geomorph.2004.09.011

831 Powrie, W. (2014). On track: the future for rail infrastructure systems. *Proceedings of the Institution*
 832 *of Civil Engineers: Civil Engineering*. 167, 4, 177-185.

833 Ridley, A.M., Dineen, K., Burland, J.B. and Vaughan, P.R. (2003). Soil matrix suction: some examples
 834 of its measurement and application in geotechnical engineering. *Géotechnique*. 53, 2, 241 – 254.

835 Reid, M. E., Baum, R. L., Lattusen, R. G., and Ellis, W. L. (2008). Capturing landslide dynamics and
 836 hydrologic triggers using near-real-time monitoring. *Landslides and Engineered Slopes*. Eds: Chen et
 837 al. Taylor and Francis, London. Vol. 1, 179-191.

838 Richards, D.J., Chandler, R.J. and Lock, A.C. (2003). Electronic data transfer systems for field
 839 monitoring. *Proceedings of the Institution of Civil Engineers: Geotechnical Engineering*. 156, 1, 47-55.

840 Sadler, J., Griffin, D., Gilchrist, A., Austin, J., Kit, O., and Heavisides, J. (2016). GeoSRM - online
 841 geospatial safety risk model for the GB rail network. *IET Intelligent Transport Systems*. 10, 1, 17-24.
 842 10.1049/iet-its.2015.0038

843 Silvast, M., Nurmikolu, A., Wiljanen, B. and Levomaki, M. (2013). Identifying frost-susceptible areas
 844 on Finnish railways using the ground penetrating radar technique. *Proceedings of the Institution of*
 845 *Mechanical Engineers Part F: Journal of Rail and Rapid Transit*. 227, 3-9.

846 Scaioni, M., Longoni, L., Melillo, V., Papini, M. (2014). Remote sensing for landslide investigations: an
 847 overview of recent achievements and perspectives. *Remote Sensing*. 6, 1-26.

848 Smethurst, J. A., Briggs, K. M., Powrie, W., Ridley, A. and Butcher, D. J. E. (2015). Mechanical and
 849 hydrological impacts of tree removal on a clay fill railway embankment. *Géotechnique*, 65, 11, 869-
 850 882. doi:10.1680/geot.14.p.010.

851 Smethurst, J. A., Clarke, D. and Powrie, W. (2006). Seasonal changes in pore water pressure in a
 852 grass-covered cut slope in London Clay. *Géotechnique*. 56, 8, 523-537. doi:
 853 10.1680/geot.2006.56.8.523.

854 Smethurst, J. A., Clarke, D. and Powrie, W. (2012). Factors controlling the seasonal variation in soil
 855 water content and pore water pressures within a lightly vegetated clay slope. *Géotechnique*. 62, 5,
 856 429-446. doi:10.1680/geot.10.p.097.

857 Smethurst, J. A. and Powrie, W. (2007). Monitoring and analysis of the bending behaviour of discrete
 858 piles used to stabilise a railway embankment. *Géotechnique*. 57, 8, 663-677.
 859 doi:10.1680/geot.2007.57.8.663.

860 Smith, A. and Dixon, N. (2015). Quantification of landslide velocity from active waveguide generated
861 acoustic emission. *Canadian Geotechnical Journal*, 52, 4, 413-425. doi:10.1139/cgj-2014-0226

862 Smith, A., Dixon, N., Meldrum, P. and Haslam, E. (2014). Inclinator casings retrofitted with
863 acoustic real-time monitoring systems. *Ground Engineering*. October issue.

864 Smith, A., Dixon, N., Moore, R. and Meldrum, P. (2016). Acoustic emission monitoring of coastal
865 slopes in north-east England, United Kingdom. Submitted to *QJEGH* (to accompany this paper).

866 Smith, A., Dixon, N., Meldrum, P., Haslam, E. and Chambers, J. (2014). Acoustic emission monitoring
867 of a soil slope: comparisons with continuous deformation measurements. *Géotechnique Letters*. 4, 4,
868 255-261. doi:10.1680/geolett.14.00053

869 Springman, S. M., Askarinejad, A., Casini, F., Friedel, S., Kienzler, P., Teyssiere, P. and Thielen, A.
870 (2012). Lessons learnt from field tests in some potentially unstable slopes in Switzerland. *Acta*
871 *Slovenica Geotechnica*. 1, 5-29.

872 Springman, S.M., Kienzler, P., Casini, F. and Askarinejad, A. (2009). Landslide triggering experiment in
873 a steep forested slope in Switzerland. *17th International Conference on Soil Mechanics &*
874 *Geotechnical Engineering ICSMGE*, Alexandria, Egypt, Vol. 2, 1698-1701.

875 Stähli, M., Sättele, M., Huggel, C., McArdell, B.W., Lehmann, P., Van Herwijnen, A., Berne, A.,
876 Schleiss, M., Ferrari, A., Kos, A., Or, D., and Springman, S.M. (2014). Review article: Monitoring and
877 prediction in Early Warning Systems (EWS) for rapid mass movements. *Natural Hazards and Earth*
878 *System Sciences (NHES)*. 2, 7149-7179.

879 Steelman, C. M., Endres, A. L. and Jones, J. P. (2012). High-resolution ground-penetrating radar
880 monitoring of soil moisture dynamics: field results, interpretation, and comparison with unsaturated
881 flow model. *Water Resources Research*. 48, 1–17. doi: 10.1029/2011WR011414

882 Tang, A.M., Askarinejad, A., Brencic, M., Cui, Y.J., Diez, J.J., Firgi, T., Gajewska, B., Gentile, F., Grossi,
883 G., Jommi, C., Kehagia, F., Koda, E., ter Maat, H.W., Lenart, S., Lourenco, S., Oliveira, M., Osinski, P.,
884 Springman, S.M., Stirling, R., Toll, D.G., Van Beek, V., Hughes, P. N., Dijkstra, T.A. (2016). Atmosphere
885 – vegetation – soil interactions in a climate change context; changing conditions impacting on
886 engineered transport infrastructure slopes. Submitted to *Quarterly Journal of Engineering Geology*
887 *and Hydrogeology*.

888 Thévenaz, L. (2010). Brillouin distributed time-domain sensing in optical fibers: state of the art and
889 perspectives. *Frontiers of Optoelectronics in China*. 3, 1, 13–21. doi: 10.1007/s12200-009-0086-9

890 Toll, D. G., Lourenco, S. D. N., Mendes, J., Gallipoli, D., Evans, F. D., Augarde, C. E., Cui, Y. J., Tang, A.
891 M., Rojas, J. C., Pagano, L., Mancuso, C., Zingariello, C. and Tarantino, A. (2011). Soil suction
892 monitoring for landslides and slopes. *Quarterly Journal of Engineering Geology and Hydrogeology*.
893 44, 1, 23-33. doi: 10.1144/1470-9236/09-010

894 Toll, D.G., Lourenço, S. D. N. and Mendes, J. (2013). Advances in suction measurements using high
895 suction tensiometers. *Engineering Geology*. 165, 29–37. doi: 10.1016/j.enggeo.2012.04.013

896 Topp, G., David, J., and Annan, A. (1980). Electromagnetic determination of soil water content:
897 measurements in coaxial transmission lines. *Water Resources Research*. 16, 3, 574-582.

- Uhlemann, S. S., Sorensen, J. P. R., House, A. R., Wilkinson, P. B., Roberts, C., Gooddy D. C., Binley A. M. and Chambers, J. E. (2016). Integrated time-lapse geoelectrical imaging of wetland hydrological processes. *Water Resources Research*. 52, 3, 1607-1625.
- Utili, S., Castellanza, R., Galli, A. and Sentenac, P. (2015). Novel approach for health monitoring of earthen embankments. *Journal of Geotechnical and Geoenvironmental Engineering*. 141, 3. doi: 10.1061/(ASCE)GT.1943-5606.0001215.
- Wang, E. F., Wang, G., Zhang, Y. M., Huo, Z. T., Peng, X. M., Araiba, K. and Takeuchi, A. (2008). Displacement monitoring on Shuping landslide in the Three Gorges Dam Reservoir area, China from August 2004 to July 2007. *Landslides and Engineered Slopes*. Eds: Chen et al. Taylor and Francis, London. Vol. 2, pp. 1321 – 1327.
- Ward, W. O. C., Wilkinson, P. B., Chambers, J. E., Oxby, L. S. and Bai, L. (2014). Distribution-based fuzzy clustering of electrical resistivity tomography images for interface detection. *Geophysical Journal International*, 197, 1, 310-321
- Wasowski, J., Bovenga, F., Dijkstra, T., Meng, X., Nutricato, R. and Chiaradia, M. T. (2014). Persistent scatterers interferometry provides insight on slope deformations and landslide activity in the mountains of Zhouqu, Gansu, China. *Proceedings of World Landslide Forum 3*, 2-6 June 2014, Beijing.
- Westerberg, B., Bertilsson, R., and Lofroth, H. (2016). Monitoring of negative pore water pressure in silt slopes. Submitted to *QJEGH* (to accompany this paper).
- Westerberg, B., Bertilsson, R., Prästings, A., Müller, R., Bengtsson, P. E. (2014). *Negative pore water pressures and stability of silt slopes* (in Swedish). Swedish Geotechnical Institute, Publication 9.
- Wilkinson, P. B., Chambers, J. E., Meldrum, P. I., Gunn, D. A., Ogilvy, R. D. and Kuras, O. (2010). Predicting the movements of permanently installed electrodes on an active landslide using time-lapse geoelectrical resistivity data only. *Geophysics Journal International*. 183, 2, 543–556.
- Wilkinson, P. B., Uhlemann, S., Chambers, J. E., Meldrum, P. I., and Loke, M. H. (2015). Development and testing of displacement inversion to track electrode movements on 3-D electrical resistivity tomography monitoring grids. *Geophysics Journal International*. 200, 3, 1566–1581. doi: 10.1093/gji/ggu483
- Zhu, H.-H., Shi, B., Yan, J.-F., Zhang, J. and Wang, J. (2015). Investigation of the evolutionary process of a reinforced model slope using a fiber-optic monitoring network. *Engineering Geology*. 186, 34-43.

Figure captions

Figure 1. BIONICS research embankment, Northumberland, UK. The facility has been used to understand earthworks behaviour in relation to climate and test new instrumentation approaches. (Photo curtesy of Dr Ross Stirling, Newcastle University, UK).

Figure 2. Norwegian XGEO system, showing colour coded landslide hazard determined from rain and snowmelt, and soil saturation data. The hazard map is updated four times a day.

Figures



Figure 1. BIONICS research embankment, Northumberland, UK. The facility has been used to understand earthworks behaviour in relation to climate and test new instrumentation approaches. (Photo curtesy of Dr Ross Stirling, Newcastle University, UK).

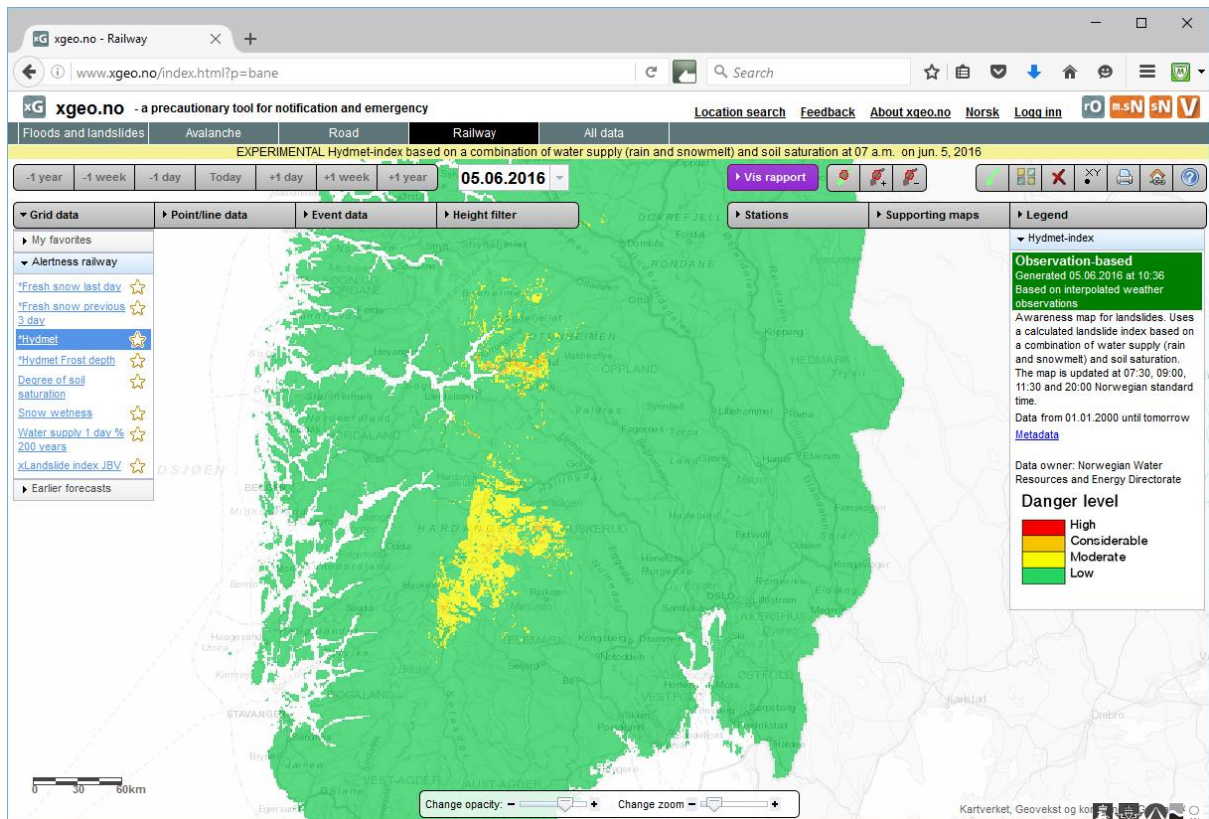


Figure 2. Norwegian XGEO system, showing colour coded landslide hazard determined from rain and snowmelt, and soil saturation data. The hazard map is updated four times a day.

Tables

Table 1: Recorded earthwork failures in the UK Network Rail system 2003 to 2014 (re-drawn from Abbott *et al.*, 2014).

	Embankments	Soil cuttings	Rock cuttings
Number of formally recorded failures	307	485	488
Number of derailments caused by earthworks failure	2	11	4
Average probability of derailment given failure (%)	0.7	2.3	2.1

Table 2: Applications for instrumentation and monitoring of infrastructure slopes

Application	Objectives of monitoring scheme	What to monitor? And number of instruments	Frequency of readings, and duration of monitoring	Analysis and interpretation of data	Example case histories
i) Monitoring the condition of problem slopes (including earthworks that are subject to significant changes in loading or profile, and ensuring function of remedial measures)	<ul style="list-style-type: none"> To understand the depth and extent of an existing failure, and the conditions (such as pore water pressure) that may have caused it To ensure that any ongoing displacements of materials that have already slipped remain small To demonstrate if remedial works are (or are not) required To check the performance of remedial measures 	<ul style="list-style-type: none"> Displacements with depth using inclinometers; pore water pressure using piezometers; weather/climate For large areas of instability, ground surface displacements may be monitored using large-area approaches such as satellite based LiDAR 	<ul style="list-style-type: none"> If the hazard posed by the slope is low, and initial displacements small, readings could be relatively infrequent, and data may not need to be logged continuously Duration may depend on hazard posed; may be months or years if monitoring is needed to limit risk to infrastructure 	<ul style="list-style-type: none"> Readings may be plotted and analysed on a periodic basis, for example once a week or month 	Monitoring of earthwork pore water pressures and displacements (Hughes <i>et al.</i> , 2016; Smethurst <i>et al.</i> , 2015)

ii) Obtaining parameters for use in design of remedial schemes	<ul style="list-style-type: none"> To understand the depth and extent of an existing failure, and ground water conditions 	<ul style="list-style-type: none"> Displacements with depth using inclinometers; pore water pressure using piezometers; weather/climate parameters (precipitation, temperature) 	<ul style="list-style-type: none"> If the hazard posed by the slope is low, readings could be relatively infrequent, e.g. monthly. Duration needs to be sufficient to make a reasonable assessment of extent of failure and likely worst pore water pressure conditions 	<ul style="list-style-type: none"> Readings can be plotted and analysed on a periodic basis, for example once a week or month 	For example, in stabilisation of earthworks using piles, see O'Kelly <i>et al.</i> (2008), and Smethurst and Powrie (2007)
iii) Early warning systems to provide alarm of actual failure, or indication of incipient failure	<ul style="list-style-type: none"> To warn of actual or incipient failure that may pose a direct risk to safety of transport systems 	<ul style="list-style-type: none"> Displacement is the obvious indicator of incipient failure in many non-brittle materials; commonly assessed using inclinometers or tilt meters. Climate, pore water pressures/suctions and soil moisture content may be secondary indicators 	<ul style="list-style-type: none"> Frequency of readings may be high, to attempt to assess risk in 'real time' if failure may occur rapidly. This would lean towards in-ground instrumentation, or tilt meters fixed to points on the slope surface, that are continuously datalogged. 	<ul style="list-style-type: none"> Data may need to be interpreted rapidly, and in-part by machine (computer or datalogger), assessing monitoring data against pre-determined thresholds 	A review of early warning systems is given by Stahl <i>et al.</i> (2014). Details of a system using acoustic emission monitoring in a cutting slope are given by Dixon <i>et al.</i> (2015)
iv) Monitoring slopes to manage risk at the infrastructure corridor scale	<ul style="list-style-type: none"> To investigate changes in key parameters along significant lengths of asset To warn of incipient failure that may pose a direct risk to safety 	<ul style="list-style-type: none"> Large numbers of instruments may be used along significant lengths of transport corridor The need to contain cost leads to measurements of ground surface displacement, or near-surface changes in pore water pressure/ suction, or parameters such as soil moisture content. Ground surface displacements may be monitored using large-area approaches such as satellite based LiDAR 	<ul style="list-style-type: none"> Frequency of readings may be high (every few minutes), if there is a need to assess risk in real time because failure may occur rapidly Condition monitoring may take place over many years 	<ul style="list-style-type: none"> Large volumes of data may need to be interpreted rapidly, and thus likely in-part by machine (computer or datalogger), assessing monitoring data against pre-determined thresholds 	Uttil <i>et al.</i> (2014) describe the use of monitoring information to consider the stability of longer lengths of asset. An overview for consideration of slopes at the corridor scale is given by Dijkstra <i>et al.</i> (2014)
v) Research: monitoring slopes to understand mechanisms of degradation and failure	<ul style="list-style-type: none"> To investigate particular modes of deterioration or failure To investigate processes (such as changes in pore water pressure) that lead to 	<ul style="list-style-type: none"> A wide range of instrumentation may be used, including more unusual types to determine less commonly measured parameters (e.g. permeability) 	<ul style="list-style-type: none"> Frequency of readings from instruments is likely to be high (hourly or sub-hourly), to obtain high-quality temporal data-sets. Duration of monitoring may 	<ul style="list-style-type: none"> Readings may be collected and analysed infrequently, to the needs of the research programme 	Examples include: <ul style="list-style-type: none"> Long-term variations of pore water pressure (Smethurst <i>et al.</i>, 2012; Glendinning <i>et al.</i>, 2014) Investigations of extreme

	failure	<ul style="list-style-type: none"> Instrumentation may be extensive to obtain a detailed profile of variation with, for example, depth 	be long, to assess, for example, long-term changes in pore water pressures over several years of climate		<p>wet winter pore water pressures (Briggs <i>et al.</i>, 2013)</p> <ul style="list-style-type: none"> Investigation of suctions supporting silt / silty sandy slopes (Casini <i>et al.</i>, 2013; Westerberg <i>et al.</i>, 2014, 2016) Controlled failure of a full-scale test embankment (Lehtonen <i>et al.</i>, 2015) Understanding rainfall infiltration driven failure (Akca <i>et al.</i>, 2011; Askarinejad <i>et al.</i>, 2012)
vi) Development and testing of new types of instrumentation	<ul style="list-style-type: none"> To understand the performance of new instrumentation systems Calibration and validation of instruments 	<ul style="list-style-type: none"> A mix of conventional and new instruments 	<ul style="list-style-type: none"> Frequency of readings is likely to be high (hourly or sub-hourly), to obtain high-quality temporal data-sets Duration of monitoring may be longer, if new instrumentation needs to be proved in full range of conditions 	<ul style="list-style-type: none"> Readings may be collected and analysed infrequently, to the needs of the research programme 	<p>Research sites such as Hollin Hill, North Yorkshire, UK (Chambers <i>et al.</i>, 2011) and Nafferton embankment, Northumberland, UK (Glendinning <i>et al.</i>, 2014) are being used to assess the performance of new monitoring instruments and techniques.</p> <p>Examples of new instrumentation include moisture and displacement monitoring using electrical resistivity tomography (ERT) (Chambers <i>et al.</i>, 2014; Gunn <i>et al.</i>, 2015; Lehmann <i>et al.</i>, 2013; Wilkinson <i>et al.</i>, 2010), and movement monitoring using acoustic emission (AE) monitoring (Smith <i>et al.</i>, 2014b)</p>

Table 3: New monitoring technologies

Instrument/ technique	Description	Accuracy	Resolution		Other Notes
			Temporal	Spatial	
Surface deformation monitoring					
Global positioning system (GPS)	<ul style="list-style-type: none">GPS system receives time signals from orbiting satellites and positioning is based on signal travel times.Minimum of 4 satellites required for position calculation (i.e. x, y, and z) accurate to ~15 m.Accuracy improvements can be achieved by using:<ul style="list-style-type: none">Differential GPS: correction of atmospheric disturbances from comparison of GPS position with known fixed position of a base station; accuracy ~ 0.1 mReal Time Kinematic (RTK) GPS: for positioning the carrier phase of the signal is used rather than the actual time signal; accuracy < 0.01 mAccuracy of RTK-GPS is required for monitoring of mass movements (Millis <i>et al.</i>, 2008).	m to mm	<ul style="list-style-type: none">Low: manual repeated surveysHigh: continuous monitoring on spatially fixed receivers	<ul style="list-style-type: none">High: depending on number of monitoring locations	<ul style="list-style-type: none">Dependent on satellite coverage; reception limited in strong topographic depressions (e.g. alpine valleys)Affected by signal scattering: limited accuracy in forested areasConsiderable time requirement for full site surveysInstrumentation and processing software are of high costPermanent installations have been used to monitor movements of large natural landslides causing damage to transport infrastructure (e.g. Massey <i>et al.</i>, 2013)Most likely to be used for applications i, iii, iv and v in Table 2
Photogrammetry	<ul style="list-style-type: none">3D reconstruction of surface topography from overlapping photographs taken from different positions (at least 2).Accuracy mainly dependent on photograph resolution and number of overlapping photographs (i.e. number of shot positions per covered area, Bemis <i>et al.</i>, 2014).Both aerial (e.g. using manned or unmanned aircraft/aerial vehicles) and terrestrial photogrammetry can be used.	m to mm	<ul style="list-style-type: none">Low: restricted by time requirements for photograph acquisition and data processingHigh: continuous monitoring on permanently installed cameras	<ul style="list-style-type: none">High: high accuracy point cloud/DEM, deformation monitoring for entire study site	<ul style="list-style-type: none">Application limited by high cost and time requirementsPost-processing of data relatively complex (e.g. see Akca <i>et al.</i>, 2011)Widely used for digital terrain mapping and monitoring surface change for natural rock slopes and landslides; a small number of examples of application to infrastructure slopes (e.g. Jang <i>et al.</i>, 2008)Most likely to be used for applications i, iv, and v in Table 2

Remote sensing	<ul style="list-style-type: none"> • Terrestrial-, aerial-, or satellite-based recording of reflected electromagnetic energy from the earth's surface. • Typical examples used in investigations of surface deformation (Petley <i>et al.</i>, 2005; Scaioni <i>et al.</i>, 2014): <ul style="list-style-type: none"> ○ LiDAR (Light Detection and Ranging): distance measurement employing backscattered energy of laser beam, used to create Digital Elevation Models ○ InSAR (Interferometric synthetic aperture radar): Mapping of phase differences between reflected radar waves of different acquisition times, representative of surface deformation. 	m to mm	<ul style="list-style-type: none"> • Medium to low: restricted by time required for survey (i.e. in case of terrestrial and aerial surveys) and processing 	<ul style="list-style-type: none"> • High: high accuracy point cloud/DEM, deformation monitoring for entire study site 	<ul style="list-style-type: none"> • Application limited by high cost and time requirements (i.e. terrestrial and aerial surveys) • Post-processing of data relatively complex • Temporal resolution dependent on satellite orbit (i.e. time between repeated data acquisition over same location) • Accuracy dependent on signal wavelength and atmospheric condition • Positioned reflectors may be required to overcome seasonal changes in vegetation • Aerial surveys (e.g. Miller <i>et al.</i>, 2012) have been used to characterise and look at longer duration changes within infrastructure earthworks • Most likely to be used for applications i, iv, and v in Table 2
Fibre optics (e.g. Brillouin Optical Time Domain Reflectometry – BOTDR)	<ul style="list-style-type: none"> • Determination of locally applied strain to a single optical fibre cable by time-domain analysis of frequency spectra of backscattered light pulses (Thévenaz, 2010). • Frequency shifts caused by changes in fibre density. • Time-domain analysis allows for determination of strain/deformation location. • Other optical fibre strain measurement approaches can be used, e.g. Bragg gratings (Glisic and Inaudi, 2007) 	<p>Strain measurement: 0.2%</p> <p>(e.g. 2 mm for 1m spatial resolution)</p>	<ul style="list-style-type: none"> • High: continuous monitoring of permanent installations 	<ul style="list-style-type: none"> • High: cable layout can be adapted to site conditions to optimize coverage and resolution 	<ul style="list-style-type: none"> • No absolute measure for displacements • Relatively high cost • Need for correction of temperature effects • Complex processing required • Can also be used subsurface, such as in a borehole • Widely used to measure strain in structural elements, but no known applications to unreinforced transport infrastructure slopes • Most likely to be used for applications i, iii, and iv in Table 2
Accelerometer, Geophone	<ul style="list-style-type: none"> • Recording of ground surface acceleration in response to: <ul style="list-style-type: none"> ○ Earthquakes (i.e. as trigger for slope destabilisation) ○ Rapid (i.e. brittle) landslide movements • Acceleration usually measured employing spring-mounted magnetic masses moving within wire coils generating electric signals. Microchip MEMS accelerometers are widely used. 	Acceleration: 0.1 m/s²	<ul style="list-style-type: none"> • High: continuous monitoring of permanently installed sensors 	<ul style="list-style-type: none"> • Low to high: dependent on number and distribution of accelerometers or geophones 	<ul style="list-style-type: none"> • Recording of movement changes only; limited detection capability of low velocity ductile movements (e.g. creep) • Extraction of movement periods from background noise may be difficult • Requires complex post-processing • No known applications for transport infrastructure slopes • Most likely to be used for applications i and iii in Table 2

Electrode tracking using electrical resistivity monitoring	<ul style="list-style-type: none"> Resistivity measurements are sensitive to the subsurface resistivity distribution and electrode separations Monitoring installations usually consist of either a line or grid of electrodes, with electrode spacing ranging from 0.5m to 5.0m Measured resistivities can be inverted to track electrode, and thus landslide movement (Wilkinson <i>et al.</i>, 2010; Wilkinson <i>et al.</i>, 2015), along a line or surface grid 	5% to 10% of electrode spacing (e.g. 0.025 m to 0.5 m, dependent on electrode layout)	<ul style="list-style-type: none"> Medium to high: dependent on measurement layout; 2D lines can be measured hourly, 3D grids usually daily 	<ul style="list-style-type: none"> Medium to high: dependent on measurement layout 	<ul style="list-style-type: none"> Accuracy dependent on resistivity data quality Other data streams required to calibrate/confirm measurements Requires complex installation and post-processing High cost measurement system The approach has been demonstrated using an installation installed within a natural landslide (Wilkinson <i>et al.</i>, 2010) Most likely to be used for applications i and v in Table 2
Subsurface deformation monitoring					
Time domain reflectometry (TDR)	<ul style="list-style-type: none"> Deployment of coaxial cables (or optical; see BOTDR) in vertical boreholes Measurement of reflections along a conductor Localized deformation of coaxial cable leads to local impedance contrast at which a pulse is reflected Time-domain analysis allows for determination of deformation location Rate of impedance change is indirectly proportional to ground movement rate (Kane <i>et al.</i>, 2001; Millis <i>et al.</i>, 2008) 	cm to mm (dependent on cable length)	<ul style="list-style-type: none"> Low: manual surveys using portable pulse generators High: continuous monitoring of permanently installed systems 	<ul style="list-style-type: none"> Low to medium: depending on whether used in single borehole or borehole network 	<ul style="list-style-type: none"> No direct measurements of deformation or deformation rate Costs range from low (infrequent, manual surveys) to high (continuous, permanent monitoring or borehole network) Sold as a commercial system, and has been installed into numerous natural and engineered slopes (Kane <i>et al.</i>, 2001) Most likely to be used for applications i, ii, iii, and v in Table 2
Shape Acceleration Array (SAA)	<ul style="list-style-type: none"> Comprises a string of MEMS (micro-electro-mechanical systems) sensors, installed inside boreholes Sensors are placed at regular intervals. Each sensor measures 3-dimensional displacements (Abdoun <i>et al.</i>, 2013) 	± 1.5 mm per 30 m array length	<ul style="list-style-type: none"> High: continuous monitoring 	<ul style="list-style-type: none"> Low to medium: depending on whether used in single borehole or borehole network 	<ul style="list-style-type: none"> Instrumentation and processing software are of high cost SAA string can be retrieved from the borehole Can provide early warning of slope instability Care should be taken with processing software (Buchli <i>et al.</i>, 2016) Sold as a commercial system and has been used quite widely in stable and unstable infrastructure slopes (e.g. Dixon <i>et al.</i>, 2015) Most likely to be used for applications i, ii, iii, and v in Table 2
Active waveguide and Slope ALARMS sensor (i.e. acoustic emission monitoring)	<ul style="list-style-type: none"> Comprises a steel waveguide (i.e. as conductor for acoustic emission signals) and angular granular backfill Host slope deformation causes deformation of granular backfill, creating high energy acoustic emission (AE) signals travelling along the waveguide (Dixon <i>et al.</i>, 2003) AE rates are proportional to slope movement rates, 	Differentiation of movement rates that differ by an order of magnitude (e.g. 0.01 mm/h and 0.1 mm/h)	<ul style="list-style-type: none"> High: continuous monitoring 	<ul style="list-style-type: none"> Low to medium: depending on whether used in single borehole or borehole 	<ul style="list-style-type: none"> Sensitive to slow rates and small displacements Most applicable to slopes failing along a defined shear surface Relatively low cost instrumentation Can provide early warning of slope instability Emerging technology; has been trialled in a clay cutting slope (Dixon <i>et al.</i>, 2015) and at the

	highlighting accelerations and decelerations of movements (Smith <i>et al.</i> , 2014; Smith and Dixon, 2015; Dixon <i>et al.</i> , 2015)			network	BIONICS facility (Glendinning <i>et al.</i> , 2014), with a number of other installations in natural landslides. <ul style="list-style-type: none"> Most likely to be used for applications i and iii in Table 2
Electrical resistivity tomography (ERT)	<ul style="list-style-type: none"> ERT measurements consist of electrodes placed at the surface and/or in boreholes Resistivity is sensitive to the subsurface lithology, e.g. clay content; inverted resistivity models represent a volumetric image of the local lithology Temporal changes in the resistivity distribution can inform about mass movements. Changes can be quantified using emerging boundary extraction algorithms (e.g. Chambers <i>et al.</i>, 2015; Uhlemann <i>et al.</i>, 2016) 	m to cm dependent on data quality and depth of changes	<ul style="list-style-type: none"> Medium to high: varies between daily and hourly, depending on measurement layout 	<ul style="list-style-type: none"> Medium to high: depending on measurement layout (i.e. 2D or 3D acquisition) 	<ul style="list-style-type: none"> Measurement sensitivity reduced with increasing distance to electrodes Complex installation and processing required Used to measure ground movements for a range of applications, including natural landslides; no known applications to transport infrastructure slopes Most likely to be used for applications i, ii and v in Table 2
Subsurface condition monitoring					
Conventional soil moisture probes	<ul style="list-style-type: none"> Based on relative permittivity measurements, which is related to moisture content using Topps equation (Topp <i>et al.</i>, 1980) Main techniques: <ul style="list-style-type: none"> Time-domain reflectometry (TDR): Relative permittivity derived from the travel time of an electro-magnetic pulse through a waveguide Capacitance sensors: Relative permittivity determined based on the charging time of a capacitor, employing the soil as dielectric 	Relative permittivity: ± 1 Moisture content: $\pm 3\%$ of measurement	<ul style="list-style-type: none"> High: continuous monitoring on permanently deployed sensors 	<ul style="list-style-type: none"> Low to medium: sensor samples only surrounding medium, can be increased if used in sensor networks 	<ul style="list-style-type: none"> Moisture content derived through empirical relationships Usually requires calibration Robust and reliable sensor technology Latest developments include web-based real-time delivery of multi-location moisture data from sensor networks at field sites Several commercially available devices; quite widely used to measure soil moisture content in the near-surface zone of infrastructure slopes (e.g. Smethurst <i>et al.</i>, 2012; Glendinning <i>et al.</i>, 2014) Most likely to be used for applications i, iii, iv and v in Table 2
Electrical resistivity tomography (ERT) monitoring of soil moisture	<ul style="list-style-type: none"> The resistivity of a formation depends mainly on its mineralogy and degree of saturation Laboratory-derived relationships can be used to translate resistivity into moisture content Repeated ERT surveys on permanently installed electrodes can be used to image volumetric moisture movements (e.g. Chambers <i>et al.</i>, 2014; Gunn <i>et al.</i>, 2015) ERT could also be used to monitor cavity development 	Moisture content: $< \pm 5\%$	<ul style="list-style-type: none"> Medium to high: varies between daily and hourly, depending on measurement layout 	<ul style="list-style-type: none"> Medium to high: depending on measurement layout (i.e. 2D or 3D acquisition) 	<ul style="list-style-type: none"> Measurement sensitivity reduced with increasing distance between electrodes Complex installation and processing required Measurement accuracy dependent on resistivity data quality Several installations have been used to image moisture changes in clay infrastructure slopes (Glendinning <i>et al.</i>, 2014; Gunn <i>et al.</i>, 2015). Many other examples of use in natural slopes. Most likely to be used for applications i, iv and v in Table 2

High-capacity pore water suction probes	<ul style="list-style-type: none"> Probes consist of <ul style="list-style-type: none"> Filter, acting as interface between soil and measurement device Water reservoir Pressure measuring device Recent improvements of measurement range and accuracy through reduction of water reservoir and higher air entry pressures of the ceramic filter (Toll <i>et al.</i>, 2011, 2013) Allowing for suction measurements ranging from 0 – 2000 kPa 	Pore water pressure/suction : $> \pm 5$ kPa	<ul style="list-style-type: none"> High: continuous monitoring of permanently installed sensors 	<ul style="list-style-type: none"> Low to high: dependent on number and distribution of probes 	<ul style="list-style-type: none"> Limited accuracy if applied at low suctions Long-term measurement drift may occur Laboratory re-saturation necessary if water reservoir dries out Probes have been trialled in a clay embankment in the UK (Toll <i>et al.</i>, 2011, 2013) Most likely to be used for applications i, iii, iv and v in Table 2
Probes for indirect measurements of pore water suction	<ul style="list-style-type: none"> Probes consist of a soil moisture device encapsulated within a porous ceramic of known water retention properties. Soil moisture in ceramic measured, and related to suction in the soil Accuracy is dependent on correct calibration between suction and moisture content of ceramic (Smethurst <i>et al.</i>, 2012) 	Pore water suction: high readings $\pm 10\%$	<ul style="list-style-type: none"> High: continuous monitoring on permanently deployed sensors 	<ul style="list-style-type: none"> Low to medium: sensor samples only surrounding medium, can be increased if used in sensor networks 	<ul style="list-style-type: none"> Requires careful calibration Generally robust sensor technology Latest developments include web-based real-time delivery of multi-location suction data from sensor networks at field sites Several commercially available devices; quite widely used to measure pore water suction in the near-surface zone of infrastructure slopes (e.g. Smethurst <i>et al.</i>, 2012; Glendinning <i>et al.</i>, 2014) Most likely to be used for applications i, iii, iv and v in Table 2
Ground penetrating radar (GPR)	<ul style="list-style-type: none"> Measurement based on the propagation of electromagnetic waves in the subsurface, i.e. wave speed dependent on dielectric properties Use on non-guided waves (in contrast to TDR where guided waves are used) Properties of reflected, ground, and cross-borehole waves can be used (Huisman <i>et al.</i>, 2003; Steelman <i>et al.</i>, 2012) GPR could also be used to characterise (Di Prinzio <i>et al.</i>, 2010) and monitor cavity development 	Moisture content: $> \pm 0.02$ m³m⁻³	<ul style="list-style-type: none"> Low to medium: manual surface or borehole surveys 	<ul style="list-style-type: none"> Medium to high: depending on measurement layout and employed frequency 	<ul style="list-style-type: none"> High cost measurement system Requires complex post-processing Limited applicability in high conductive soils (i.e. clay) due to attenuation of the GPR signal Commonly used to establish ballast depth in railway formations. Used by Donohue <i>et al.</i> (2011, 2013) to investigate an old clay railway embankment Most likely to be used for applications i, iv, and v in Table 2

