Analytical solutions for predicting tensile and in-plane shear strengths of triaxial weave fabric composites

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A R T I C L E  I N F O
Article history:
Received 19 January 2016
Revised 24 May 2016
Available online 2 May 2017

Keywords:
Composite
Triaxial weave fabric (TWF)
Strength
Tensile
In-plane shear

A B S T R A C T
This paper deals with new analytical solutions to predict tensile and in-plane shear strengths of triaxial weave fabric (TWF) composites accounting for the interaction between angularly interlacing yarns. The triaxial yarns in three directions of 0° and ±60° in micromechanical unit cell (UC) are idealized as the curved beams with a path depicted by using sinusoidal shape functions. The tensile and in-plane shear strengths of TWF composites are derived by means of the minimum total complementary potential energy principle founded on micromechanics. In order to validate the new model, the predictions are compared with experimental data in prior literatures. It is shown that the predictions from the new model agree well with experimental results.

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1. Introduction

Due to the superior specific stiffness and strength as well as excellent damage resistance and processing property, textile composites are undergoing a wide research and application in engineering structures (especially in aerospace structures) (Callus et al., 1999; Cheng and Xiong, 2009; Xiong et al., 2009; Mahadik and Hallett, 2011; Cheng et al., 2012; Bakar et al., 2013). Recently, with potential for the wider applications of triaxial weave fabric (TWF) composite structures (here the TWF means the net form with cavities between yarns), there is growing interest in assessing mechanical properties and failure mechanism for TWF composites using experimental, theoretical and numerical methods. Aoki et al. experimentally determined tensile stiffness and strength (Aoki and Yoshida, 2006), flexural stiffness (Aoki et al., 2007) and fatigue strength (Kosugi et al., 2011) of TWF composites. The experiments showed that tensile and flexural stiffness of TWF composites increased with the increasing width of specimen and eventually converged to a certain value. Fatigue strengths of TWF composites with thinner and thicker tows were almost same and fatigue failure progression of TWF composites with thicker tows was much slower as against that with thinner tows. Fatigue damage accumulation mechanism was attributed to the debonding at the tow intersection. Montesano et al. (2014) observed three main damage modes (including cracks within ±60° yarns, in resin rich zones and on yarn interfaces) and the changes of dominant cracks and crack density with stress level until final failure, by tracking the development of microscopic damage of TWF composites under axial tensile through scanning electron microscopes. Due to the resource constraints, various analytical models have been achieved to evaluate mechanical properties of TWF composites. Kueh et al. (2005), Kuth and Pellegrino (2007), and Kuth (2014) presented a transverse isotropic straight beam model to predict linear elastic response of TWF composites based on Kirchhoff plate theory. The tension, compression and shear properties were obtained by using the equivalent stiffness matrix determined from the FE method. The size effects on mechanical properties of TWF composites subjected to tensile and bending loadings were discussed. The predictions were in a good agreement with experimental data. Miravete et al. (2006) proposed an analytical meso-mechanical approach for predicting tensile modulus and strengths of TWF composites by considering the geometry, mechanical properties and the degradation of fiber and matrix. The new model was validated with numerical results and experimental data. Xu et al. (2007) developed an analytical method by using equivalent anisotropic plate theory to assess the buckling behavior of TWF composites subjected to a bi-axial compression loading. It was found that the buckling load increased with the increasing number of crossovers in TWF composites to approach a certain value. Predictions had a good correlation with the FE results. El-Hajjar et al. (2013) established discrete yarn and three-layer analytical models to evaluate the effects of braiding parameters on overall stiffness of TWF composites. Comparison between predictions and experiments showed the calculations from three-layer analytical model with the best prediction...
accuracy. Kuth (2013) presented a thin plate model to analyze the stability of sandwich columns with elastic cores of TWF composite skin-sheets subjected to uniaxial compression loading under simply support boundary condition. The effects of thickness, aspect ratio, modulus of core and imperfections in TWF composite skinsheets on critical buckling loads were discussed. In recent years, the FE technique has also received considerable attention in assessing mechanical properties and failure mechanism of TWF composites. Zhao et al. (2004) simulated progressive failure process of TWF composites subjected to axial tensile loading in terms of the FE model and maximum stress. Hoffman and Tsai-Wu failure criteria. The numerical results coincided well with experiments. Aoki and Yoshida (2006) and Aoki et al. (2007) implemented a homogenization method based on the FE analysis of the UC to predict the in-plane flexural stiffness and the thermal expansion coefficient of TWF composites with the finite width under free boundary condition. Predictions had a good correlation with experiments. Tsai et al. (2008) developed a parallelogram spring model to investigate the effects of fabric parameters on elastic properties of TWF composites. Alternatively, fiber yarns were treated as a series of curved springs only with axial rigidity and the UC of braided composites was then discretized into the FE model with spring and solid elements under proper displacement boundary conditions. The predictions correlated well with theoretical results and experiments from previous literature. Zhang and Binienda (2014) numerically investigated mechanical behaviors of TWF composites with infinite size under both axial and transverse tensile loading by using a meso-scale FE model. The free-edge effects on global stress-strain response were simulated and local failure mechanisms were discussed. The numerical results coincided well with experiments. However, in order to produce the FE models, straight inclined segments or continuous mathematical functions have been used to depict in more detail the idealized TWF geometry including yarn path and cross-sectional shape. The one draw back of the FE models is their intensity and complexity. It is obvious that the analytical methods still have consistently received interests to evaluate 

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<td>in-plane shear strength per unit length of TWF composites, N/mm</td>
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<td>T₁</td>
<td>internal torque moment on 0° yarn, N·mm</td>
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<td>U₁</td>
<td>complementary potential energy of 0° yarn in the UC</td>
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<tr>
<td>U₂</td>
<td>complementary potential energy of ±60° yarn in the UC</td>
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w: width of yarn, mm
W₀: width of the UC
Wₚ: torsional section modulus of interlacing interface between triaxial yarns, mm⁴
Xₚ₀: tensile strength of yarn in longitudinal direction, MPa
X₁: tensile strength per unit length of TWF composites, N/mm
σ₁: normal stress on 0° yarn, MPa
σ₂: normal stress on ±60° yarn, MPa
τ: interlaminar shear stress on interlacing interface between triaxial yarns, MPa
τ₁: shear force per unit length in longitudinal direction, N/mm
τ₂: shear force per unit length in transverse direction, N/mm
θ: off-axis angle of yarn, rad
ΔR₁: relative shift of the UC in the direction of external force P₁, mm
ΔR₂: relative shift of the UC in the direction of external shear force P₅₁, mm
ε: tension strain of the UC
γ: shear strain of the UC
FE: finite element
UC: unit cell
TWF: triaxial weave fabric
mechanical properties of textile composites and there is a need for a more practical and expedient analytical model for structural applications, especially in aerospace field.

In this paper, an attempt is made to develop novel analytical models to predict tensile and in-plane shear strengths of TWF composites by using the minimum total complementary potential energy principle based on apt geometrical approximation and assumptions for TWF composites. New analytical solutions for predicting tensile and in-plane shear strengths are established. The results from the models are compared with experimental data from the prior literatures.

2. Geometrical model for TWF composites

It is well known that quasi-isotropic macro-mechanical behaviors of TWF composites depend upon fabric geometry (e.g., cross-section and undulation geometry of yarn), fiber volume fraction and constituent properties, etc. In general, the TWF composites are formed by angularly interlacing and curing the preformed yarns in the directions of 0° and ±60° (shown in Fig. 1), and the crimp or undulation of yarns resulted from the angularly interlacing between triaxial yarns has a significant influence on mechanical properties and strengths of TWF composites. The periodicity of the repeating pattern in a TWF can be used to isolate a small UC (unit cell) which is sufficient to describe the fabric architecture. The UC in yarn interlacing pattern for a TWF is indicated in Fig. 2 by dark borders. From Fig. 2, it is clear that the undulated length of 0° yarn is twice those of ±60° yarns in the UC, and it can be shown that

\[ L_0 = \frac{2L}{\tan 30^\circ} = 2L\sqrt{3} \]  
\[ W_0 = 2L \]  

where \( L_0 \) and \( W_0 \) are respectively the length and width of the UC. \( L \) is the half undulated length of 0° yarn.

Fig. 3 shows the micrograph of 0° yarn obtained from scanning electron microscopes and illustrates the definitions of three directions of \( x, y \) and \( z \) axis in local coordinate system. The coordinate axes \( x, y \) and \( z \) respectively denote the longitudinal, transverse and through-thickness directions of the yarns to ensure the fibers with correct 3D orientation, i.e., to keep three directions of \( x, y \) and \( z \) axis in local coordinate system consistent with three normal stresses. In order to establish analytical models of the UC, fundamental assumptions made in this paper are as follows:

1. The yarns are idealized as the curved beams with an undulated neutral axis depicted by using a smooth and continuous sinusoidal curve function (shown in Fig. 3).
2. The flat cross-section of yarns is regarded as the rectangular section with the width of \( w \) and the thickness of \( h \) (shown in Fig. 4).
3. The interlacing interface between triaxial yarns is approximated as the rhombus section with the width of \( w \) and the length of \( b \) (shown in Fig. 5).
4. Previous literatures (Aoki and Yoshida, 2006; Kuth and Pellegriino, 2007) show that the load-strain response for TWF composites under uniaxial tensile state was linear until final brittle rupture and no obvious yield took place (see Fig. 6), whereas the load-strain response under in-plane shear state had the linear elastic and nonlinear regions until final failure, but the yield strength was close to the ultimate strength (see Fig. 7). Therefore, the failure strength in this work is defined as the critical stress at failure initiation (i.e., at initial load drop on the load-displacement response) before the decrease of modulus, and the decrease of modulus is neglected.
5. Shear force component is negligibly small in contrast to the axial force in tensile or compression case for plane woven composites, while the axial force component is negligibly small in comparison with the shear force in pure shear loading case (Potluri and Thammanandra, 2007; Naik et al., 2003).

From Assumption (1) and Fig. 3, the undulated neutral axis of triaxial yarns can be expressed by a sinusoidal function.

\[ z = \frac{h}{2} \sin \frac{\pi x}{L} \]  

where \( h \) is the thickness of yarn.

From Assumption (2) and Fig. 4, the area and inertia moments of idealized cross-section of triaxial yarns are respectively

\[ A = wh \]  
\[ I_y = \frac{wh^3}{12} \]  
\[ I_z = \frac{w^2h}{12} \]  
\[ I_p = I_y + I_z \]  

where \( A \) is cross-section area of yarn, \( I_y \), \( I_z \) and \( I_p \) are respectively the inertia moment of yarn along transverse and through-thickness directions of yarn. \( I_p \) is the polar inertia moment.

From Assumption (3) and Fig. 5, the torsional section modulus of rhombic interlacing interface between triaxial yarns can be written as

\[ W_p = \alpha wb^2 \]  
with

\[ b = \frac{w}{\cos 30^\circ} = \frac{2w\sqrt{3}}{3} \]
where $W_p$ is the torsional section modulus, $w$ is the width of yarn, $b$ is the length of rhombic interlacing interface between triaxial yarns. $\alpha$ is the correct coefficient, for the rhombus section, $\alpha = 0.1181$.

Substituting Eq. (9) into Eq. (8) deduces

$$W_p = \frac{4}{3} \alpha w^3$$

(10)

3. Analytical solution for tensile strength

As shown in Fig. 8, in the case of external uniaxial tensile loading $P_t$, there are equivalent external axial forces $F_1$ and $F_2$ on 0° and ±60° yarns in the UC. In order to obtain the equivalent external axial forces $F_1$ and $F_2$ on 0° and ±60° yarns, with the aid of force decomposition as a vector, it is possible to have

$$P_t = 2F_1 + 2F_2 \sin 30^\circ$$

(11)

or

$$F_2 = P_t - 2F_1$$

(12)

where $P_t$ is the external uniaxial tensile force. $F_1$ and $F_2$ are respectively the equivalent external axial forces on 0° and ±60° yarns.

Due to the rotationally symmetry of the UC, only half UC necessitates to be analyzed. Figs. 9a and b respectively illustrate the
internal force and bending moments on 0° and ±60° yarns (where \( N \) is the internal interaction force at the centre of interlacing interface between triaxial yarns along through-thickness direction. \( M_1 \) and \( M_2 \) respectively represent the internal bending moments on 0° and ±60° yarns). In terms of moment equilibrium of half UC, one has

\[
M_1 = 0
\]

(13)

From Fig. 9a, for a differential segment \( dx \) on 0° yarn, by means of Eq. (3), the tangent of off-axial angle can be shown to be

\[
\tan \theta = \frac{dz}{dx} = \frac{\pi h}{2L} \cos \left( \frac{\pi x}{L} \right)
\]

(14)

where \( \theta \) is the off-axial angle of yarn.

The internal force and bending moment on any cross-section of 0° yarn can be expressed as

\[
F(x) = F_1 \cos \theta = \frac{F_1}{\sqrt{1 + \tan^2 \theta}} \quad (0 \leq x \leq \frac{L}{2})
\]

(15)

\[
M(x) = \frac{h F_1}{2} \sin \left( \frac{\pi x}{L} \right) - \frac{N}{2} x \quad (0 \leq x \leq \frac{L}{2})
\]

(16)

Substituting Eq. (14) into Eq. (15) results in

\[
F(x) = \frac{F_1}{\sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2}} \quad (0 \leq x \leq \frac{L}{2})
\]

(17)

By analogy aid of Eqs. (16) and (17), it is possible to have the internal force and bending moment on any cross-section of ±60° yarn as

\[
F(x) = \frac{P_i - 2F_1}{\sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2}} \quad (0 \leq x \leq L)
\]

(18)

\[
M(x) = \left( \frac{P_i}{2} - F_1 \right) \left( 1 - \cos \left( \frac{\pi x}{L} \right) \right) - \frac{N}{2} x - M_2 \quad (0 \leq x \leq L)
\]

(19)

It goes without saying that there exist three undetermined forces and moments of \( F_1, N \) and \( M_2 \) in Eqs. (16)-(19), and it is feasible to determine these undetermined forces and moments in view of the principle of minimum total complementary potential energy. The complementary potential energies of 0° and ±60° yarns in the UC are respectively

\[
U^*_1 = \frac{2}{EA} \int_0^{L/2} M^2(x) \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \ dx
\]

(20)

\[
U^*_2 = \frac{1}{EA} \int_0^L M^2(x) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \ dx
\]

(21)

where \( U^*_1 \) and \( U^*_2 \) are respectively the complementary potential energies of 0° and ±60° yarns in the UC. \( E \) is the elastic modulus of yarn in longitudinal direction. Note that the square root factors in Eqs. (20) and (21) are the transformation coefficients for infinitesimal element length \( dx \) mapped to the neutral axis of curved beam.

Substituting Eqs. (16)-(19) into Eqs. (20) and (21), it can be shown that

\[
U^*_1 = (B_1 + B_4) F_1^2 + B_2 N^2 + B_4 F_1 N
\]

(22)

\[
U^*_2 = (B_5 + B_{11}) \left( \frac{P_i}{2} - F_1 \right)^2 + B_6 N^2 + B_7 M_2^2 + B_8 N \left( \frac{P_i}{2} - F_1 \right)
\]

\[+B_9 M_2 \left( \frac{P_i}{2} - F_1 \right) + B_{10} N M_2
\]

(23)

where \( B_i \) \((i=1, 2, \ldots, 11)\) are the transformation variables and are defined in Appendix A.

Because the complementary potential energies of −60° and 60° yarns are same, from Eqs. (22) and (23), total complementary potential energy of the UC can be shown to be

\[
\Pi^* = 2U^*_1 + 4U^*_2 = 2(B_1 + B_4) F_1^2 + 4(B_5 + B_{11}) \left( \frac{P_i}{2} - F_1 \right)^2
\]

\[+(2B_2 + 4B_6) N^2
\]

\[+4B_7 M_2^2 + 2B_3 F_1 N + 4B_8 N \left( \frac{P_i}{2} - F_1 \right) + 4B_9 M_2 \left( \frac{P_i}{2} - F_1 \right)
\]

\[+4B_10 N M_2
\]

(24)

By minimizing the total complementary potential energy principle of the UC and taking transformation leads to

\[
\begin{cases}
2(B_1 + B_4) F_1 + (B_3 - 2B_8) N - 2B_6 M_2 = 2(B_5 + B_{11})\bar{R} \\
(B_4 - 2B_7) \bar{F}_1 + 2B_3 F_1 N + 2B_10 M_2 = -B_8 \bar{R} \\
-2B_9 \bar{F}_1 + 4B_8 N + 4B_7 M_2 = -B_9 \bar{R}
\end{cases}
\]

(25)

Solving Eq. (25) by using Cramer’s rule shows

\[
F_1 = C_1 \bar{R}
\]

(26)

\[
N = C_2 \bar{R}
\]

(27)

\[
M_2 = C_3 \bar{R}
\]

(28)

where \( C_i \) \((i=1, 2, 3)\) are the transformation variables and are defined in Appendix B.

With the aid of the potential energy principle, the relative shift \( \Delta \bar{R} \) of the UC in the direction of external force \( \bar{F}_1 \) is deduced as

\[
\Delta \bar{R} = \frac{\partial \Pi^*}{\partial \bar{F}_1} = 4(B_1 + B_4) \bar{C}_1 \bar{F}_1 + 8(B_5 + B_{11})(0.5 - C_1) \bar{F}_1
\]

\[+4(B_2 + 2B_6) \bar{C}_2 \bar{R} + 8B_9 \bar{C}_1 \bar{R}
\]

\[+4B_3 C_1 \bar{R} + 8B_8 C_2 (0.5 - C_1) \bar{R} + 8B_8 C_3 (0.5 - C_1) \bar{R}
\]

\[+8B_10 C_3 \bar{R}
\]

(29)
In light of the definition of strain, tension strain $\varepsilon$ of the UC is
\[
\varepsilon = \frac{\Delta L}{2L} \tag{30}
\]
And the tension force per unit length on the UC is determined as
\[
\sigma_1 = \frac{P_1}{2\sqrt{3}L} \tag{31}
\]
On the basis of engineering beam theory, maximum normal stresses at the path peaks and valleys of 0° and ±60° yarns in the UC resulted from internal force and bending moment can be respectively written as
\[
\sigma_{1\text{max}} = \frac{F_1}{A} + \frac{NLh}{8L_y} \tag{32}
\]
\[
\sigma_{2\text{max}} = \frac{F_2}{A} + \frac{NLh + 2M_jh}{4L_y} \tag{33}
\]
where $\sigma_1$ and $\sigma_2$ are respectively the normal stresses on 0° and ±60° yarns.
Substituting Eqs. (12), (26)–(28) into Eqs. (32) and (33) yields
\[
\sigma_{1\text{max}} = R_1 \left( \frac{C_1}{A} + \frac{C_2hL}{8L_y} \right)^{-1} \tag{34}
\]
\[
\sigma_{2\text{max}} = R_1 \left( 1 - \frac{2C_1}{A} + \frac{C_2 + 2C_3 h}{4L_y} \right)^{-1} \tag{35}
\]
According to the maximum stress criterion, from Eqs. (34) and (35), critical external uniaxial tensile forces pertaining to tensile failures of 0° and ±60° yarns are respectively
\[
P_{f1} = X_0 \left( \frac{C_1}{A} + \frac{C_2hL}{8L_y} \right)^{-1} \tag{36}
\]
\[
P_{f2} = X_0 \left( 1 - \frac{2C_1}{A} + \frac{C_2 + 2C_3 h}{4L_y} \right)^{-1} \tag{37}
\]
where $P_{f1}$ and $P_{f2}$ are respectively the critical external uniaxial tensile forces pertaining to tensile failures of 0° and ±60° yarns. $X_0$ is the tensile strength of yarn in longitudinal direction.
From Eqs. (36) and (37), critical external uniaxial tensile force pertaining to tensile failure of TWF composites is obtained as
\[
P_f = \min \{ P_{f1}, P_{f2} \} \tag{38}
\]
where $P_f$ is the critical external uniaxial tensile force pertinent to tensile failure of TWF composites.
From Eqs. (1) and (38), the tensile strength per unit length of TWF composites is attained as
\[
X_t = \frac{P_f}{L_0} = \frac{\min \{ P_{f1}, P_{f2} \}}{2\sqrt{3}} \tag{39}
\]
where $X_t$ is the tensile strength per unit length of TWF composites.

4. Analytical solution for in-plane shear strength

From Fig. 10, it is clear that for the UC of TWF composites subjected to external in-plane shear loading of $P_{31}$ and $P_{32}$, there exist equivalent external shear forces $F_1$ and $F_2$ on 0° and ±60° yarns. Similarly, with the aid of force decomposition, it is possible to have
\[
P_{31} = 2F_1 - 2F_2 \sin 30° \tag{40}
\]
\[
P_{32} = 2F_2 \cos 30° \tag{41}
\]
where $P_{31}$ and $P_{32}$ are respectively the external in-plane shear forces along longitudinal and transverse directions.

According to moment equivalence of the UC, one has
\[
F_1 = 2F_2 \tag{42}
\]
Substituting Eq. (42) into Eqs. (40) and (41) yields
\[
F_1 = \frac{2}{3}P_{31} \tag{43}
\]
From Eqs. (1) and (2), the in-plane shear forces per unit length on the UC in longitudinal and transverse directions are respectively
\[
\tau_1 = \frac{P_{31}}{L_0} = \frac{P_{31}}{2\sqrt{3}L} \tag{44}
\]
\[
\tau_2 = \frac{P_{32}}{W_0} = \frac{P_{32}}{2L} \tag{45}
\]
where $\tau_1$ and $\tau_2$ respectively denote the in-plane shear forces per unit length on the UC in longitudinal and transverse directions.
Substituting Eqs. (40)–(42) into Eqs. (44) and (45) results in
\[
\tau_1 = \frac{F_2 \sqrt{3}}{2L} \tag{46}
\]
\[
\tau_2 = \frac{F_2 \sqrt{3}}{2L} \tag{47}
\]
From Eqs. (46) and (47), it is clear that the in-plane shear force per unit length on the UC in longitudinal direction is equal to that in transverse direction. This implies that the micro-mechanical curved beam model of yarns in the UC follows the equivalent law of shear stress.
Figs. 11a and b respectively demonstrate the internal restraining forces and moments on 0° and 60° yarns (where $N_1$ is the internal interactive force at the intersecting points between triaxial yarns along through-thickness direction. $N_2$ and $N_3$ are the restraining forces from yarn's rotation along through-thickness direction at the intersecting points between triaxial yarns to depict the shearing action at the interface area. $T_1$, $T_2$, $M_1$ and $M_2$ are respectively the internal torque and bending moments on 0° and 60° yarns). From Fig. 11a, it is possible to have the internal bending and torque moments at any cross-section of 0° yarn as
\[
M_y(x) = \begin{cases} 
N_1 x - N_3 \sin \frac{x}{2} 
\frac{N_1}{4} - N_3 \sin \frac{x}{2} + \frac{N_3 b \sqrt{3}}{8} (2 \sin \frac{x}{2} - \sqrt{3}) & (0 \leq x < \frac{1}{2}) \\
\frac{N_1}{4} (2x + b \sqrt{3}) + \frac{b}{8} (2x + b \sqrt{3}) & (\frac{1}{2} \leq x < 1) 
\end{cases} \tag{48}
\]
\[
M_x(x) = \begin{cases} 
M_1 - F_1 x + \frac{N_3 y}{8} \left( 2x \sqrt{3} + b \right) - \frac{b}{8} (2x + b \sqrt{3}) & (0 \leq x < \frac{1}{2}) \\
M_1 - F_1 x + \frac{N_3 y}{8} (2x \sqrt{3} + b) + \frac{b}{8} (2x + b \sqrt{3}) & (\frac{1}{2} \leq x < 1) 
\end{cases} \tag{49}
\]
It stands to reason that there are seven undetermined forces and moments $N_1, N_2, N_3, M_1, M_2, T_1$ and $T_2$ in Eqs. (48)–(53). Using the same method as in the above section, these undetermined forces and moments can be determined by resorting to the principle of minimum complementary energy. The complementary potential energies of $0^\circ$ and $60^\circ$ yarns in the UC are respectively

$$U_1^c = \frac{2}{E_y} \int_0^\frac{L}{2} M_1^2(x) \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx + \frac{2}{E_L} \int_0^\frac{L}{2} M_2^2(x) \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx + \frac{2}{G_T} \int_0^\frac{L}{2} T^2(x) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \, dx$$

$$U_2^c = \frac{1}{E_y} \int_0^L M_1^2(x) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \, dx + \frac{1}{E_L} \int_0^L M_2^2(x) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \, dx + \frac{1}{G_T} \int_0^L T^2(x) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \, dx$$

where $G$ is the in-plane shear modulus of yarn.

Substituting Eqs. (48)–(53) into Eqs. (54) and (55) leads to

$$U_1^c = Q_1 F_1^2 + Q_2 M_1^2 + Q_3 T_1^2 + Q_4 N_1^2 + Q_5 N_2^2 + Q_6 N_3^2 + Q_7 F_1 T_1 + Q_8 F_1 N_1 + Q_9 F_2 N_2 + Q_{10} T_1 N_1 + Q_{11} T_1 N_2 + Q_{12} M_1 N_2 + Q_{13} M_1 N_3 + Q_{14} T_1 N_3 + Q_{15} T_2 N_1 + Q_{16} T_2 N_2 + Q_{17} T_2 N_3 + Q_{18} N_1 N_3 + Q_{19} N_2 N_3$$

$$U_2^c = Q_{20} F_2^2 + Q_{21} M_2^2 + Q_{22} T_2^2 + Q_{23} N_1^2 + Q_{24} N_2^2 + Q_{25} N_3^2 + Q_{26} F_2 M_2 + Q_{27} F_2 T_2 + Q_{28} F_2 N_1 + Q_{29} F_2 N_2 + Q_{30} F_2 N_3 + Q_{31} M_2 N_2 + Q_{32} M_2 N_3 + Q_{33} T_1 N_1 + Q_{34} T_1 N_2 + Q_{35} T_1 N_3 + Q_{36} N_1 N_2 + Q_{37} N_1 N_3 + Q_{38} N_2 N_3$$

where $Q_i (i=1, 2, ..., 38)$ are the transformation variables and are defined in Appendices C–E.

From Eqs. (56) and (57), the total complementary potential energy of the UC becomes

$$\prod \sum 2 Q_1 F_1^2 + 2 Q_2 M_1^2 + 2 Q_3 T_1^2 + 4 Q_4 N_1^2 + 4 Q_5 M_1^2 + 4 Q_6 M_2^2 + (2 Q_4 + 4 Q_23) N_1^2 + (2 Q_5 + 4 Q_24) N_2^2 + (2 Q_6 + 4 Q_25) N_3^2 + 2 Q_7 F_1 T_1 + 2 Q_8 F_1 N_1 + 2 Q_9 F_2 N_2 + 2 Q_{10} T_1 N_1 + 2 Q_{11} T_1 N_2 + 2 Q_{12} M_1 T_1 + 2 Q_{13} M_1 N_2 + 2 Q_{14} T_1 N_3 + 2 Q_{15} T_2 T_1 + 2 Q_{16} T_2 N_1 + 2 Q_{17} T_2 N_2 + 2 Q_{18} T_2 N_3$$
By minimizing total complementary potential energy of the UC and taking transformation, it can be shown that

$$
\begin{bmatrix}
M_1 \\
T_1 \\
M_2 \\
T_2 \\
N_1 \\
N_2 \\
N_3
\end{bmatrix}
= -F_1
$$

By numerically solving Eq. (59), one has

$$
N_2 = C_4 F_1
$$

$$
N_3 = C_5 F_1
$$

where $C_4$ and $C_5$ are the transformation variables.

Substituting Eq. (43) into Eq. (59), and then substituting Eq. (59) into Eq. (58), the total complementary potential energy of the UC can be expressed with the external shear loading $P_{17}$. Again, using the same method as in the above section (or in accordance with the potential energy principle), the relative shift $\Delta N_{31}$ of the RUC in the direction of the external shear loading $P_{31}$ is determined as

$$
\Delta N_{31} = \frac{\partial \Pi}{\partial P_{31}}
$$

In interest of the definitions of shear strain, the shear strain $\gamma$ of the TWF composites is

$$
\gamma = \frac{\Delta N_{31}}{2T_3}
$$

In reality, for the UC of TWF composites subjected to external in-plane shear loading, there exists an interlaminar shear stress on the interlacing interface between triaxial yarns and the shear debonding failure always occurs at the interlacing interface between triaxial yarns in TWF composites due to the lower interlaminar shear strength (Kosugi et al., 2011). Hereby, the in-plane shear properties and strength of TWF composites are dependent on the interlaminar shear strength of interfacing interface between triaxial yarns. From Fig. 11, the internal torsion moment on the interlacing interface between triaxial yarns along through-thickness direction resulted from the restraining forces $N_2$ and $N_3$ can be written as

$$
T_3 = bN_2 + N_3 \sqrt{3}
$$

where $T_3$ is the internal torque moment on the interlacing interface between triaxial yarns resulting from restraining forces of $N_2$ and $N_3$.

Substituting Eqs. (60) and (61) into Eq. (64) results in

$$
T_3 = bF_1(C_4 + C_5 \sqrt{3})
$$

According to mechanics of materials, maximum interlaminar shear stress on the interlacing interface between triaxial yarns is determined as

$$
\tau_{\text{max}} = \frac{T_3}{W_p}
$$

where $\tau$ is the interlaminar shear stress on interfacing interface between triaxial yarns.

Substituting Eqs. (10) and (65) into Eq. (66) leads to

$$
\tau_{\text{max}} = \frac{3bF_1}{4\alpha w^3}(C_4 + C_5 \sqrt{3})
$$

According to the maximum stress criterion, from Eq. (67), critical equivalent external force on 0° yarn pertinent to interlaminar shear failure of interlacing interface between triaxial yarns can be shown to be

$$
\frac{3bF_1}{4\alpha w^3}(C_4 + C_5 \sqrt{3}) = S_0
$$

necessarily,

$$
F_{17} = \frac{4S_0 w^3}{3b(C_4 + C_5 \sqrt{3})}
$$

where $F_{17}$ is the critical equivalent external force on 0° yarn pertinent to interlaminar shear failure of interlacing interface between triaxial yarns. $S_0$ is the interlaminar shear strength of yarn.

From Eqs. (43), (46) or (47) and (69), the in-plane shear strength per unit length of TWF composites becomes

$$
S = \sqrt{3} \frac{F_{17}}{4T_3} = \frac{4\sqrt{3}S_0 w^3}{3b(T_4 + C_5 \sqrt{3})}
$$

where $S$ is the in-plane shear strength per unit length of TWF composites.

5. Comparisons between predictions and experiments

In order to validate new analytical models presented in this paper, it is essential to compare the predictions determined from the new models with the experiments. Previous literatures reported the fabric specifications and mechanical properties (shown in Table 1) and the experiments of tensile and in-plane shear strengths (shown in Figs. 6 and 7 and Table 2) for three types of TWF composites (i.e., T300/NM35 [Aoki and Yoshida, 2006; Aoki et al., 2007], T300/Hexel 8552 [Kuth and Pellegrino, 2007; Zhao, 2010] and another carbon fiber/epoxy resin [Zhao et al., 2004]).

From the fabric specifications and mechanical properties listed in Table 1, by using Eqs. (30), (31), (39), (44), (63) and (70), the tensile and in-plane shear forces per unit length versus strain curves and the tensile and in-plane shear strengths of three types of TWF composites are respectively predicted (shown in Figs. 6 and 7, and Table 2). From Figs. 6 and 7, it is obvious that the predicting curves correlate very well with experiments in the linear elastic range before the decrease of modulus. From Table 2, it is evident that the maximum relative deviations of predictions for tensile and in-plane shear strengths from experiments are respectively 3.4% and 20.44%, with an acceptable scatter. The reason for 20.44% in the difference between the predicted and experimental values is probably that the predictions are obtained from Assumption 4 (namely, the decrease of modulus is neglected). However, the load-strain response under in-plane shear state had the linear elastic and nonlinear regions until final failure (see Fig. 7). This results in the predicted shear strengths in this work (i.e., at initial load drop on the shear load-displacement response before the decrease of modulus) being stiffer than those shown in the experimental ones.

From mentioned above, it is argued that the new curved beam-based models presented in this paper are a valid and rational basis for tensile and in-plane shear strength analysis of TWF composites. Using these models, tensile and in-plane shear strengths of TWF composites could be predicted without any additional fabric level experimental investigation, i.e. only the input of basic properties of
Acknowledgment

This project was supported by NSFC (No. 51375033 and 51405006)

Appendix A. Definitions for transformation variables \( B_i \)

\[
B_1 = \frac{h^2}{2EA} \int_0^L \frac{1}{x} \left( \sin \frac{\pi x}{L} \right)^2 \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \quad (A-1)
\]

\[
B_2 = \frac{1}{2EL_y} \int_0^L x \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \quad (A-2)
\]

\[
B_3 = -\frac{h}{EL_y} \int_0^L \frac{x \sin \frac{\pi x}{L}}{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \quad (A-3)
\]

Appendix B. Definitions for transformation variables \( C_i \)

\[
C_1 = \frac{4(B_5 + B_{11})}{(2B_3 - 4B_8)} - \frac{4B_9}{4B_{10}} - \frac{4B_8}{4B_{10}} - 2B_8 - 9B_7 \quad (B-1)
\]

\[
C_2 = \frac{4(B_1 + B_4) + 8(B_5 + B_{11})}{(2B_3 - 4B_8)} - \frac{4B_9}{4B_{10}} - \frac{4B_8}{4B_{10}} - 2B_8 - 9B_7 \quad (B-2)
\]
\[
Q_1 = 2(K_{11} + K_{17} + K_{27} + K_{37}) \quad (C-1)
\]
\[
Q_2 = 2(K_{10} + K_{16}) \quad (C-2)
\]
\[
Q_3 = 2(K_{26} + K_{36}) \quad (C-3)
\]
\[
Q_4 = 2(K_1 + K_4 + K_{28} + K_{38}) \quad (C-4)
\]
\[
Q_5 = 2(K_6 + K_{19} + K_{40}) \quad (C-5)
\]
\[
Q_6 = 2(K_3 + K_5 + K_{12} + K_{18} + K_{29} + K_{39}) \quad (C-6)
\]
\[
Q_7 = 2(K_{13} + K_{20}) \quad (C-7)
\]
\[
Q_8 = 2(K_{30} + K_{41}) \quad (C-8)
\]
\[
Q_9 = 2(K_{33} + K_{45}) \quad (C-9)
\]
\[
Q_{10} = 2(K_{24} + K_{47}) \quad (C-10)
\]
\[
Q_{11} = 2(K_{15} + K_{23} + K_{34} + K_{46}) \quad (C-11)
\]
\[
Q_{12} = 2K_{22} \quad (C-12)
\]
\[
Q_{13} = 2(K_{14} + K_{21}) \quad (C-13)
\]
\[
Q_{14} = 2(K_{31} + K_{42}) \quad (C-14)
\]
\[
Q_{15} = 2K_{44} \quad (C-15)
\]
\[
Q_{16} = 2(K_{32} + K_{45}) \quad (C-16)
\]
\[
Q_{17} = 2(K_8 + K_{49}) \quad (C-17)
\]
\[
Q_{18} = 2(K_2 + K_7 + K_{35} + K_{48}) \quad (C-18)
\]
\[
Q_{19} = 2(K_9 + K_{25} + K_{50}) \quad (C-19)
\]
\[
Q_{20} = J_8 + J_{12} + J_{18} + J_{28} + J_{31} + J_{37} + J_{47} \quad (C-20)
\]
\[
Q_{21} = J_8 + J_{11} + J_{17} \quad (C-21)
\]
\[
Q_{22} = J_{27} + J_{30} + J_{36} + J_{46} \quad (C-22)
\]
\[
Q_{23} = J_1 + J_2 + J_{32} + J_{38} + J_{48} \quad (C-23)
\]
\[
Q_{24} = J_6 + J_{20} + J_{50} \quad (C-24)
\]
\[
Q_{25} = J_3 + J_5 + J_{13} + J_{19} + J_{39} + J_{49} \quad (C-25)
\]
\[
Q_{26} = J_{10} + J_{14} + J_{21} \quad (C-26)
\]
\[
Q_{27} = J_{29} + J_{33} + J_{40} + J_{51} \quad (C-27)
\]
\[
Q_{28} = J_{35} + J_{43} + J_{55} \quad (C-28)
\]
\[
Q_{29} = J_{35} + J_{57} \quad (C-29)
\]
\[
Q_{30} = J_{16} + J_{24} + J_{44} + J_{56} \quad (C-30)
\]
\[
Q_{31} = J_{23} \quad (C-31)
\]
\[
Q_{32} = J_{15} + J_{22} \quad (C-32)
\]
\[
Q_{33} = J_{34} + J_{41} + J_{52} \quad (C-33)
\]
\[
Q_{34} = J_{44} \quad (C-34)
\]
\[
Q_{35} = J_{42} + J_{53} \quad (C-35)
\]
\[
Q_{36} = J_{59} \quad (C-36)
\]
\[
Q_{37} = J_4 + J_{45} + J_{58} \quad (C-37)
\]
\[
Q_{38} = J_7 + J_{26} + J_{60} \quad (C-38)
\]
\[ K_6 = \frac{3h^2}{64EI_y} \int_0^\frac{\pi}{L} \left( 2 \sin \frac{\pi x}{L} - \sqrt{3} \right)^2 \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-6)

\[ K_7 = \frac{hL}{6EI_y} \int_0^\frac{\pi}{L} \sin \frac{\pi x}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-7)

\[ K_8 = \frac{hL\sqrt{3}}{12EI_y} \int_0^\frac{\pi}{L} \left( 2 \sin \frac{\pi x}{L} - \sqrt{3} \right)^2 \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-8)

\[ K_9 = \frac{h^2\sqrt{3}}{16EI_y} \int_0^\frac{\pi}{L} \left( 2 \sin \frac{\pi x}{L} - \sqrt{3} \right)^2 \times \left( \sin \frac{\pi x}{L} \right)^2 \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-9)

\[ K_{10} = \frac{1}{EI_y} \int_0^\frac{\pi}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-10)

\[ K_{11} = \frac{1}{EI_y} \int_0^\frac{\pi}{L} x^2 \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-11)

\[ K_{12} = \frac{3}{4EI_y} \int_0^\frac{\pi}{L} x^2 \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-12)

\[ K_{13} = -\frac{2}{EI_y} \int_0^\frac{\pi}{L} x \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-13)

\[ K_{14} = \frac{\sqrt{3}}{EI_y} \int_0^\frac{\pi}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-14)

\[ K_{15} = -\frac{\sqrt{3}}{EI_y} \int_0^\frac{\pi}{L} x^2 \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-15)

\[ K_{16} = \frac{1}{EI_y} \int_0^\frac{\pi}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-16)

\[ K_{17} = \frac{1}{EI_y} \int_0^\frac{\pi}{L} x^2 \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-17)

\[ K_{18} = \frac{1}{16EI_y} \int_0^\frac{\pi}{L} (2x + b\sqrt{3})^2 \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-18)

\[ K_{19} = \frac{1}{16EI_y} \int_0^\frac{\pi}{L} (2x + b\sqrt{3})^2 \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-19)

\[ K_{20} = -\frac{2}{EI_y} \int_0^\frac{\pi}{L} x \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-20)

\[ K_{21} = \frac{1}{2EI_y} \int_0^\frac{\pi}{L} (2x + b\sqrt{3}) \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-21)

\[ K_{22} = \frac{1}{2EI_y} \int_0^\frac{\pi}{L} (2x + b\sqrt{3}) \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-22)

\[ K_{23} = -\frac{1}{2EI_y} \int_0^\frac{\pi}{L} x(2x + b\sqrt{3}) \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-23)

\[ K_{24} = -\frac{1}{2EI_y} \int_0^\frac{\pi}{L} x(2x + b\sqrt{3}) \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-24)

\[ K_{25} = \frac{1}{8EI_y} \int_0^\frac{\pi}{L} (2x + b\sqrt{3})(2x + b\sqrt{3}) \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-25)

\[ K_{26} = \frac{1}{GI_p} \int_0^\frac{\pi}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-26)

\[ K_{27} = \frac{h^2}{4GI_p} \int_0^\frac{\pi}{L} \sin^2 \frac{\pi x}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-27)

\[ K_{28} = \frac{h^2}{4GI_p} \int_0^\frac{\pi}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-28)

\[ K_{29} = \frac{3h^2}{16GI_p} \int_0^\frac{\pi}{L} \sin^2 \frac{\pi x}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-29)

\[ K_{30} = \frac{h}{GI_p} \int_0^\frac{\pi}{L} \sin \frac{\pi x}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-30)

\[ K_{31} = -\frac{b}{GI_p} \int_0^\frac{\pi}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-31)

\[ K_{32} = -\frac{h\sqrt{3}}{2GI_p} \int_0^\frac{\pi}{L} \sin \frac{\pi x}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-32)

\[ K_{33} = -\frac{bh}{2GI_p} \int_0^\frac{\pi}{L} \sin \frac{\pi x}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-33)

\[ K_{34} = -\frac{h^2\sqrt{3}}{4GI_p} \int_0^\frac{\pi}{L} \sin \frac{\pi x}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-34)

\[ K_{35} = \frac{bh\sqrt{3}}{4GI_p} \int_0^\frac{\pi}{L} \sin \frac{\pi x}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-35)

\[ K_{36} = \frac{1}{GI_p} \int_0^\frac{\pi}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-36)

\[ K_{37} = \frac{h^2}{4GI_p} \int_0^\frac{\pi}{L} \sin \frac{\pi x}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-37)

\[ K_{38} = \frac{h^2}{4GI_p} \int_0^\frac{\pi}{L} \sin \frac{\pi x}{L} \sqrt{1 + \left( \frac{\pi h}{2L} \cos \frac{\pi x}{L} \right)^2} \, dx \]  
(D-38)
Appendix E. Definitions for transformation variables $\bar{J}_i$

\[ J_1 = \frac{1}{36E_L} \int_{\pi/2}^{\pi} (6x - L)^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_2 = \frac{1}{36E_L} \int_{\pi/2}^{\pi} (6x - 5L)^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_3 = \frac{h^2}{16E_L} \int_{\pi/2}^{\pi} (\cos \frac{\pi x}{L})^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_4 = \frac{h}{12E_L} \int_{\pi/2}^{\pi} (\cos \frac{\pi x}{L}) (6x - 5L) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_5 = \frac{h^2}{16E_L} \int_{\pi/2}^{\pi} (\cos \frac{\pi x}{L})^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_6 = \frac{3h^2}{64E_L} \int_{\pi/2}^{\pi} (\sqrt{3 - 2\cos \frac{\pi x}{L}})^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_7 = \frac{h^2\sqrt{3}}{16E_L} \int_{\pi/2}^{\pi} (\sqrt{3 - 2\cos \frac{\pi x}{L}}) (\cos \frac{\pi x}{L}) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_8 = \frac{1}{E_L} \int_{0}^{\pi/2} \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_9 = \frac{1}{E_L} \int_{0}^{\pi/2} x^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_{10} = \frac{2}{E_L} \int_{0}^{\pi/2} x \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_{11} = \frac{1}{E_L} \int_{0}^{\pi/2} \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_{12} = \frac{1}{E_L} \int_{0}^{\pi/2} x^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_{13} = \frac{1}{16E_L} \int_{0}^{\pi} (2x\sqrt{3} + b - L\sqrt{3})^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
\[ J_{14} = \frac{2}{E_L} \int_{0}^{\pi/2} \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]
\[ J_{15} = \frac{1}{2EL} \int_{\frac{x}{L}}^{\frac{\pi}{2L}} \left(2x\sqrt{3} + b - L\sqrt{3}\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-15)

\[ J_{16} = -\frac{1}{2EL} \int_{\frac{x}{L}}^{\frac{\pi}{2L}} x(2x\sqrt{3} + b - L\sqrt{3}) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-16)

\[ J_{17} = \frac{1}{EL} \int_{\frac{x}{L}}^{\pi L} \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-17)

\[ J_{18} = \frac{1}{EL} \int_{\frac{x}{L}}^{\pi L} x^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-18)

\[ J_{19} = \frac{1}{16EL} \int_{\frac{x}{L}}^{\pi L} \left(2x\sqrt{3} + b - L\sqrt{3}\right)^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-19)

\[ J_{20} = \frac{1}{144EL} \int_{\frac{x}{L}}^{\pi L} \left(6x + 3b\sqrt{3} - 5L\right)^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-20)

\[ J_{21} = -\frac{2}{EL} \int_{\frac{x}{L}}^{\pi L} \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-21)

\[ J_{22} = \frac{1}{EL} \int_{\frac{x}{L}}^{\pi L} \left(2x\sqrt{3} + b - L\sqrt{3}\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-22)

\[ J_{23} = \frac{1}{6EL} \int_{\frac{x}{L}}^{\pi L} \left(6x + 3b\sqrt{3} - 5L\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-23)

\[ J_{24} = -\frac{1}{2EL} \int_{\frac{x}{L}}^{\pi L} x(2x\sqrt{3} + b - L\sqrt{3}) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-24)

\[ J_{25} = -\frac{1}{6EL} \int_{\frac{x}{L}}^{\pi L} x(6x + 3b\sqrt{3} - 5L) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-25)

\[ J_{26} = \frac{1}{24EL} \int_{\frac{x}{L}}^{\pi L} \left(24x\sqrt{3} + 12b - 12L\sqrt{3}\right)(6x + 3b\sqrt{3} - 5L) \times \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-26)

\[ J_{27} = \frac{1}{GL} \int_{0}^{\frac{x}{L}} \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-27)

\[ J_{28} = \frac{h^2}{4GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right)^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-28)

\[ J_{29} = \frac{h}{GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-29)

\[ J_{30} = \frac{1}{GL} \int_{0}^{\frac{x}{L}} \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-30)

\[ J_{31} = \frac{h^2}{4GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right)^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-31)

\[ J_{32} = \frac{h^2}{4GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-32)

\[ J_{33} = \frac{h}{GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-33)

\[ J_{34} = \frac{b}{GL} \int_{0}^{\frac{x}{L}} \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-34)

\[ J_{35} = \frac{bh}{2GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right)^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-35)

\[ J_{36} = \frac{1}{GL} \int_{0}^{\frac{x}{L}} \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-36)

\[ J_{37} = \frac{h^2}{4GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right)^2 \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-37)

\[ J_{38} = \frac{9b^2}{4GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-38)

\[ J_{39} = \frac{3h^2}{16GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-39)

\[ J_{40} = \frac{h}{GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-40)

\[ J_{41} = -\frac{3b}{2GL} \int_{0}^{\frac{x}{L}} \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-41)

\[ J_{42} = \frac{h\sqrt{3}}{2GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-42)

\[ J_{43} = -\frac{3bh}{2GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-43)

\[ J_{44} = \frac{h^2\sqrt{3}}{4GL} \int_{0}^{\frac{x}{L}} \left(1 - \cos \frac{\pi x}{L}\right) \sqrt{1 + \left(\frac{\pi h}{2L} \sin \frac{\pi x}{L}\right)^2} \, dx \]  
(E-44)
\[ J_{45} = -\frac{3bh\sqrt{3}}{4Gt_p} \int_{\frac{L}{2}}^{L} \left( \cos \frac{\pi x}{L} \right) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \, dx \]  
(E-45)

\[ J_{46} = \frac{1}{Gt_p} \int_{\frac{L}{2}}^{L} \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2 \, dx \]  
(E-46)

\[ J_{47} = \frac{h^2}{4Gt_p} \int_{\frac{L}{2}}^{L} \left( 1 - \cos \frac{\pi x}{L} \right) \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2 \, dx \]  
(E-47)

\[ J_{48} = \frac{4b^2}{Gt_p} \int_{\frac{L}{2}}^{L} \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2 \, dx \]  
(E-48)

\[ J_{49} = \frac{3h^2}{16Gt_p} \int_{\frac{L}{2}}^{L} \left( \cos \frac{\pi x}{L} \right)^2 \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2 \, dx \]  
(E-49)

\[ J_{50} = \frac{h^2}{64Gt_p} \int_{\frac{L}{2}}^{L} \left( \sqrt{3} - 2 \cos \frac{\pi x}{L} \right) \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2 \, dx \]  
(E-50)

\[ J_{51} = \frac{h}{Gt_p} \int_{\frac{L}{2}}^{L} \left( 1 - \cos \frac{\pi x}{L} \right) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \, dx \]  
(E-51)

\[ J_{52} = -\frac{4b}{Gt_p} \int_{\frac{L}{2}}^{L} \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2 \, dx \]  
(E-52)

\[ J_{53} = \frac{h\sqrt{3}}{2Gt_p} \int_{\frac{L}{2}}^{L} \left( \cos \frac{\pi x}{L} \right) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \, dx \]  
(E-53)

\[ J_{54} = \frac{h}{4Gt_p} \int_{\frac{L}{2}}^{L} \left( \sqrt{3} - 2 \cos \frac{\pi x}{L} \right) \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2 \, dx \]  
(E-54)

\[ J_{55} = \frac{2bh}{Gt_p} \int_{\frac{L}{2}}^{L} \left( 1 - \cos \frac{\pi x}{L} \right) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \, dx \]  
(E-55)

\[ J_{56} = \frac{h^2\sqrt{3}}{4Gt_p} \int_{\frac{L}{2}}^{L} \left( 1 - \cos \frac{\pi x}{L} \right) \left( \cos \frac{\pi x}{L} \right) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \, dx \]  
(E-56)

\[ J_{57} = -\frac{h^2}{8Gt_p} \int_{\frac{L}{2}}^{L} \left( -\cos \frac{\pi x}{L} \right) \left( \sqrt{3} - 2 \cos \frac{\pi x}{L} \right) \right) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \, dx \]  
(E-57)

\[ J_{58} = -\frac{bh\sqrt{3}}{Gt_p} \int_{\frac{L}{2}}^{L} \left( \cos \frac{\pi x}{L} \right) \sqrt{1 + \left( \frac{\pi h}{2L} \sin \frac{\pi x}{L} \right)^2} \, dx \]  
(E-58)

References


