

1 **Landward shifts of the maximum accretion zone in the tidal**
2 **reach of the Changjiang Estuary following construction of the**
3 **Three Gorges Dam**

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15

16 **Abstract**

17 Impacts from anthropogenic activities have substantially modified the geomorphology of most of the
18 world's large rivers. While many studies have focused on the fluvially-dominated and estuarine/delta
19 segments of these rivers, their tidal reaches that links the river to the estuarine delta is much less
20 extensively documented. Yet, morphological variations in these key transition zones, however,
21 directly affect the transfer of water and sediment to the sea, and have a significant influence on the
22 delta environment. Here, we analyze the morphological variation of the Datong-Xuliujing Reach
23 (DXR), the tidal reach of the Changjiang River, following the closure and operation of the Three
24 Gorges Dam, using a unique dataset combining surveys in 1992, 2002, 2008 and 2013. The results
25 demonstrate that the DXR exhibits three different morphological development phases. When
26 sediment supply is high (at 3.18×10^8 t/y), the DXR experienced deposition (1992-2002) with the
27 maximum accretion zone located in the middle portions of the reach. Thereafter (2002-2008) the
28 channel underwent a major period of erosion coincident with a substantial decline of fluvial sediment
29 supply (to 1.72×10^8 t/y). More recently (i.e., during 2008-2013), the entire reach experienced
30 deposition, but with the maximum accretion zone shifting around 100 km landward (compared to its
31 position in 1992-2002), while the riverine sediment supply was further reduced to 1.30×10^8 t/y. Our
32 analytical modelling further reveals that a damped high fluvial discharge, induced by Three Gorges
33 Dam regulation, and a relatively strong water level fluctuation induced by tidal forcing in the wet
34 season, are responsible for the upstream shift in the maximum accretion zone. In addition, local
35 variations caused by sand mining and dredging generate spatial nonuniformity in the observed
36 patterns of erosion and deposition along the DXR. Such knowledge is of vital importance for the
37 sustainable management of large alluvial rivers and their tidal reaches as they respond to natural and
38 anthropogenic effects.

39 **Keywords:** accretion zone; tidal reach; Changjiang Estuary; Three Gorges Dam

40

41 **1. Introduction**

42 The world's estuarine deltas are dynamic transition zones where rivers meet the ocean (Savenije,
43 2012; Nienhuis et al., 2018). Estuarine deltas are critically important environments as they provide
44 important ecosystem services that support the lives and livelihoods of hundreds of millions of people
45 worldwide (Ericson et al., 2006; Tessler et al., 2015; Angamuthu et al., 2018). However, as low-lying
46 environments subject to the continually changing balance between competing fluvial and tidal
47 processes, there is concern that they are facing a major sustainability crisis of subsidence (Syvitski et
48 al., 2009; Lentsch et al., 2018; Dunn et al., 2019). Specifically, many of the world's estuarine deltas
49 are sinking as a result of the extraction of water, oil and gas from the subsurface (Syvitski et al., 2009;
50 Hoitink et al., 2017). Other human activities, such as the embankment of delta channels and the
51 construction of dams in the river catchments that feed them (Kondolf et al., 2014; Dunn et al., 2019)
52 are preventing the natural aggradation of delta plains, which could further exacerbate such delta
53 lowering (Paola et al., 2011; Giosan et al., 2013, 2014). Alongside the removal of sand from channels
54 to promote navigation and for use as building aggregate (Auerbach et al., 2015; Hackney et al., 2020),
55 these pressures are collectively changing the natural functioning of deltas. Indeed, the intensity of
56 human interventions across the world's estuarine deltas is now so high that the morphodynamic
57 evolution of many deltas can arguably no longer be considered natural, placing a number of deltas in
58 greater danger of inundation due to reductions in floodplain sedimentation, accelerated subsidence
59 and sea-level rise (Nicholls et al., 2016; Lentsch et al., 2018).

60 The scale of these human disturbances within, and upstream, of estuarine deltas may be expected
61 to have significant impacts. For example, the construction of reservoirs and the subsequent operation
62 of dams, may lead to seasonal changes in the delivery of freshwater flows that, in turn, lead to
63 adjustments in the function of fluvial and estuarine hydrology (Poff et al., 2007; Assani et al., 2011;

64 [Mei et al., 2015](#)). Within estuarine deltas, changes in channel (for example, due to sand mining) and
65 delta-plain morphology (for example, due to land reclamation) are also known to have non-local
66 impacts on changes in the tidal prism ([Angamuthu et al., 2018](#); [Bain et al., 2019](#)), feeding back to
67 further destabilise channel morphology. Importantly, these pressures are stressing many large rivers
68 and their estuarine deltas, including major rivers in east and south Asia such as the Mekong River in
69 Vietnam, the Ganges River in India, and the Pearl and Changjiang Rivers (the latter is the focus of
70 this study) in China. Indeed, most of the world's large rivers have now been significantly dammed in
71 their central and upper reaches, resulting in changes in hydraulics and morphological adjustments in
72 their lower reaches ([Räsänen et al., 2017](#); [Rahman et al., 2018](#); [Dunn et al., 2019](#)). Moreover, these
73 environmental stresses are expected to increase, both as a result of ongoing climate change, as well
74 as increasing population, urbanization and socio-economic change ([Darby et al., 2016](#); [Angamuthu
75 et al., 2018](#); [Dunn et al., 2019](#)). A detailed understanding of the ways in which tidal channel
76 morphology adjusts to environmental change is, therefore, an essential pre-requisite for ensuring that
77 the world's estuarine deltas can be managed in ways that increase their resilience.

78 As the longest river in Asia, the Changjiang (Yangtze) River has been subjected to extensive human
79 interference, particularly the construction of the Three Gorges Dam (TGD) in 2003, the world's
80 largest piece of hydraulic infrastructure ([Mei et al., 2015](#); [Dai et al., 2016](#)). Although many previous
81 studies have investigated the morphological response of the Changjiang since the closure of the TGD,
82 these prior works have mainly focused on either the upstream river basin that is completely dominated
83 by fluvial discharge (e.g. [Yuan et al., 2012](#); [Lai et al., 2017](#); [Mei et al., 2018](#); [Deng et al., 2019](#)), or
84 the estuarine delta that is directly and significantly affected by tidal asymmetry ([Kuang et al., 2013](#);
85 [Luan et al., 2017](#); [Zhang et al., 2018](#)). In contrast, much less information is available on the
86 erosion/accretion status of the tidally-influenced reach that links the river basin to the delta, much

87 less to consider how the combined effects of dam construction and altered flow regime have affected
88 the morphology of this critical transition zone. Located in the last 500 km of the river, the tidal reach
89 of the Changjiang is characterized by a dynamic environment which is dominated by fluvial forcing,
90 but affected by tidally-induced water level fluctuations. No long-term observations are available for
91 water and sediment discharge along this tidal reach due to its complicated hydrodynamic conditions
92 (Mei et al., 2019). The same problem exists in many other large rivers, such as the Mekong (Lauri et
93 al., 2012) and Mississippi (Wang et al., 2018), which are relatively under-studied in their lowermost
94 reaches. This situation severely hinders management of the Changjiang's estuarine delta, one of the
95 world's economically most important, because morphological processes along the tidal reach can
96 significantly affect the sediment budget of the entire system. This study directly fills this gap by
97 addressing the following specific objectives: 1) To detect the morphological changes of the tidal
98 Changjiang before and after construction of the TGD, using conventional bathymetric surveys; 2) To
99 explore, using an analytical model, how the altered spatial-temporal hydrodynamics affect the
100 observed morphological changes along the tidal Changjiang; 3) To identify the potential drivers of
101 the morphological evolution in the study area. The knowledge derived from this study can be
102 considered as an analogue for other major meso-tidal systems (such as the Mekong and Pearl) that
103 have similar seasonal flow regimes and tidal ranges, and which are facing similar environmental
104 pressures.

105

106 **2. Study Location**

107 The Changjiang (Yangtze) River basin, extending 6300 km from the Tibetan Plateau to the East
108 China Sea, covers about 20% of the total area of China (Yan et al., 2010). The specific focus of this

109 study is the 512 km-long portion between Datong and Xuliujing (117°37' E -121°59' E and 30°46'
110 N-31°00' N, [Fig. 1A](#)) (hereafter, DXR), the lowermost part of the Changjiang River that connects
111 the fluvial system to the estuarine delta. Datong, as the tidal limit during the dry season, indicates the
112 upstream most influence of tide. Downstream of Xuliujing, the Changjiang River directly branches
113 into the estuary. Therefore, the Datong-Xuliujing reach (DXR) is distinctive from both the river and
114 the estuary, with an obvious transition between the fluvially and tidally-dominant zones. Note that
115 the lowermost limit of the DXR is still over 100 km away from the East China Sea, its response to
116 the tidal forcing is mainly in terms of water level fluctuations, rather than tidal asymmetry ([Mei et al.,](#)
117 [2019](#)).

118 Over the period of 1992-2013, annually there were 9×10^{11} m³ and 2.31×10^8 t of water and
119 suspended sediment discharge, respectively, entering into the DXR through Datong ([Fig. S1](#)). Being
120 strongly affected by the Asian monsoon climate, the river's hydrological regime is highly seasonal,
121 with more than 70% of the flow discharge passing Datong during the wet season between May and
122 October ([Chen et al., 2016](#)). The river planform is characterized by the interconnection of relatively
123 straight and meandering channel patterns ([Fig. 1B](#)). The Changjiang Estuary is a meso-tidal system
124 with a mean tidal range of 2.2 m at Xuliujing ([Fig. S2](#)), which is primarily the result of the semidiurnal
125 M₂ tidal constituent, followed in importance by the S₂ and K₁ tides ([Zhang et al., 2018](#)). There is no
126 spring/neap variation in the tidal amplitude at Xuliujing, but a slight seasonal variation is observed,
127 with the tidal range in wet season (May to October) being on average 0.1 m higher than in the dry
128 season (November to April) ([Fig. S2](#)). Xuliujing is dominated by the ebb tide, with average flood and
129 ebb durations of 4.5 h and 8 h, respectively, and correspondingly, the mean flood and ebb tidal currents
130 are 0.61 m/s and 1.02 m/s. Datong and Zhenjiang mark the approximate limits of the tidal backwater
131 influence in the dry and flood seasons, respectively. The stations at Nanjing and Jiangyin represent

132 the uppermost limits to which the tidal flood currents penetrate, again in the dry and flood seasons,
133 respectively (Mei et al., 2019; the locations of these interfaces are indicated on Fig. 1B). The upper
134 limit of the DXR is located some 1200 km downstream of the TGD (see Fig. 1A for location). The
135 TGD project began in 2003; by 2009, when full operations began, the total water storage capacity
136 was ~40 billion m³, equivalent to 5 % of the Changjiang's mean annual discharge (Bao et al., 2015).

137 **3. Data collection and methods**

138 Four types of data were collected in this study: 1) topographic maps, dating from 1992, 2002, 2008
139 and 2013 (in each case the surveys on which the maps are based were undertaken during the wet
140 season, i.e. between May and August), were obtained from the Changjiang Waterway Bureau and
141 used to determine morphological changes along the DXR and to provide the geometric characteristic
142 parameters used subsequently in the analytical model. The resolution of the bathymetric data is further
143 introduced in Section 3.1; 2) Daily water and suspended sediment discharges at Datong were obtained
144 from the Changjiang Water Resources Commission for the period from 1992 to 2013 (the location of
145 Datong is indicated on Fig. 1B). Water discharge was estimated through the representative vertical
146 line method, which divides the entire cross-section into 6 bins by 5 velocity verticals following the
147 code for hydrologic data processing in China (MWRPRC, 2012). For each bin, its discharge is the
148 product of the wetted cross-sectional area and the mean flow velocity of the corresponding
149 representative line (Mei et al., 2019). A vertical velocity profile is measured with acoustic Doppler
150 current profilers (ADCP). Suspended sediment discharge was obtained as the product of the discharge
151 and suspended sediment concentration. Suspended sediment concentration in fluid is measured by
152 filtering and drying of the samples that were obtained from horizontal suspended sediment sampler
153 with a storage capacity of 1000 ml (MWRPRC, 2012); 3) Hourly tidal levels, as measured at
154 Xuliujing in 2002 and 2013, were obtained from the Changjiang Water Conservancy Committee; 4)

155 Monthly mean tidal ranges and water levels in 2002 and 2013 at Wuhu (WH), Maanshan (MAS),
156 Nanjing (NJ), Zhenjiang (ZJ), Jiangyin (JY), and Tianshenggang (TSG) were obtained from the
157 Changjiang Hydrology Bureau of the People's Republic of China (the locations of these stations are
158 indicated on Fig. 1B).

159 To address the issues outlined in Section 1, we firstly conducted an empirical analysis using repeat
160 bathymetric surveys, which enabled us to quantify the evolution of the Changjiang tidal channel's
161 morphology before and after construction of the TGD (Section 3.1). Then, we employed a well-
162 developed analytical model (Cai et al., 2013, 2016, 2019), in a range of simulation experiments to
163 further analyze how the changes in flow regime, channel morphology and tidal amplitude affect the
164 spatial patterns of tide-river hydrodynamics in the wet season (Section 3.2). By combining the data
165 obtained from both approaches we seek to link the observed changes in morphology to their causative
166 mechanisms.

167

168 ***3.1 Bathymetric change analysis***

169 To quantify the change in the morphology of the DXR before and after completion of the TGD, we
170 employed a dataset compiled from 4 topographic maps. The maps dating from 1992 and 2002
171 represent the bathymetric information before the construction of TGD, while the charts from 2008
172 and 2013 reflect the morphological response since its subsequent closure and operation. From each
173 map around 700-900 cross sections, spaced at a distance of around 400-600 m (this is equivalent to
174 one cross-section for roughly every 2 to 3 channel widths based on the minimum channel width) were
175 digitized to characterize the channel bathymetry. The cartographic data used to compile the original
176 maps were acquired using shipborne dual-frequency echo sounders for depth measurement and GPS
177 devices for positioning, with a vertical error of approximately 0.2 m and a positioning error of 1 m.

178 Such small depth and position errors highlight the high quality of the data (Dai et al., 2014). The map
179 scales range from 1:20,000 to 1:80,000 (Table S1A-D).

180 The data from the maps were transferred into depth points relative to Beijing 54 coordinates and
181 were corrected to the ‘Wusong Datum’ (which refers to the lowest water level) using ArcGIS.
182 Thereafter, the vector bathymetric point data from each map were gridded using the Kriging
183 interpolation method to generate a digital elevation model (DEM) with 50×50 m grid resolution (van
184 der Wal et al., 2002; Blott et al., 2006). Spatial variations in deposition and erosion of the river bottom
185 were then obtained by comparing the riverbed elevations of two adjacent (in time) surveys using:

$$186 \Delta h(p, t_1, t_2) = h_2(p, t_2) - h_1(p, t_1) \quad (1)$$

187 where $h_1(p, t_1)$ and $h_2(p, t_2)$ are, respectively, the riverbed elevations at times t_1 and t_2 at any
188 position $p(x, y)$. In this study, the entire 512 km length of the DXR is equally divided into 20 sub-
189 reaches, each with a downstream interval of 25.6 km. Then, the statistical characteristics of the sub-
190 reach averaged change in bed elevation, $\Delta h(p, t_1, t_2)$, within each of these downstream intervals
191 were analysed by fitting a Gaussian distribution, a typical pattern used in geoscience research
192 (Montreuil et al., 2014; Dai et al., 2018), to the long-stream variations in erosion/deposition:

$$193 f(\Delta h) = a \exp^{-(\Delta h - b)^2 / 2c^2} \quad (2)$$

194 where $f(\Delta h)$ is the probability density function of Δh , with a indicating the height of the curve’s
195 peak, b indicating the position of the center of the peak, and c indicating the standard deviation. In
196 this way the patterns of erosion and deposition in each epoch of change were represented as statistical
197 models (Gaussian models) with respect to distance along the reach.

198

199 ***3.2 Simulations of river-tide hydrodynamics***

200 To explore how the construction and operation of the TGD may affect the spatial–temporal patterns

201 of tide–river hydrodynamics in the DXR during the wet season, the variation of the residual (tide-
 202 averaged) water level slope in the momentum equation were calculated using the well-developed
 203 analytical model of Cai et al. (2013; 2016; 2019). Flow deceleration around the maximum residual
 204 water level slope should render a transition zone for processes of sediment transport and deposition,
 205 with most deposition taking place around this region and thus generating a maximum sediment
 206 deposition zone (Lamb et al., 2012). This can be attributed to the sudden decrease of velocity when
 207 the water surface transitions from a steep to more gentle gradient. The performance of this analytical
 208 model in the Changjiang Estuary has been compared with the numerical TELEMAC model that
 209 considers both the real bathymetrical condition and time varying inputs. The results of this
 210 comparison exercise showed that the two models are consistent in reproducing the seasonal behavior
 211 of tide–river dynamics and estimating the residual water level profile for a wide range of tide and
 212 river discharge scenarios (Zhang et al., 2016), demonstrating that the simpler analytical model can be
 213 used with confidence in this study.

214

215 **3.2.1. Basic equations for reproducing the residual water level profile**

216 In tidal rivers, the cross-sectional averaged residual water level can be obtained from the one-
 217 dimensional momentum equation (Savenije, 2012; Cai, 2016):

$$218 \frac{\partial U}{\partial t} + U \frac{\partial U}{\partial x} + g \frac{\partial Z}{\partial x} + \frac{gh}{2\rho} \frac{\partial \rho}{\partial x} + g \frac{U|U|}{K^2 h^{4/3}} = 0 \quad (3)$$

219 where U is the cross-sectional averaged velocity, Z is the free surface elevation, h is the water depth,
 220 g is the acceleration of gravity, t is time, ρ is the water density, x is the distance along the channel
 221 (starting from the estuary), and K is the Manning-Strickler friction coefficient. Assuming a periodic
 222 variation of flow velocity, the residual water level slope can be calculated as (Cai et al., 2019):

$$223 \frac{\partial \bar{Z}}{\partial x} = -\frac{1}{K^2} \overline{\left(\frac{U|U|}{h^{4/3}}\right)} - \frac{1}{2g} \frac{\partial \bar{U}^2}{\partial x} - \frac{1}{2\rho_0} \overline{h \frac{\partial \rho}{\partial x}} \quad (4)$$

224 where the over bars and the subscript zero, respectively, indicate the tidal average and the value at the
 225 seaward boundary. As Eq (4) indicates, the residual water level slope is governed by the residual
 226 friction, the advective acceleration, and density effects. Compared to the frictional dissipation, the
 227 contribution of advective acceleration and density effects have been shown to be rather small in the
 228 Changjiang Estuary (Savenije, 2012; Cai et al., 2019), Eq (4) can, therefore, justifiably be further
 229 integrated to:

$$230 \quad \bar{Z}(x) = - \int_0^x \frac{\partial \bar{Z}}{\partial x} dx = - \int_0^x \frac{\overline{U|U|}}{K^2 h^{4/3}} dx \quad (5)$$

231 Here we set the residual water level at the estuary mouth ($x=0$) as 0.

232

233 **3.2.2. Analytical solution for tide-river dynamics**

234 To derive the residual water level profiles in the estuary, the longitudinal variation of cross-
 235 sectional area and width are both assumed follow an exponential function (see Toffolon et al., 2006
 236 and Cai et al., 2016 for justification):

$$237 \quad \bar{A} = \bar{A}_r + (\bar{A}_0 - \bar{A}_r) \exp\left(-\frac{x}{a}\right) \quad (6)$$

$$238 \quad \bar{B} = \bar{B}_r + (\bar{B}_0 - \bar{B}_r) \exp\left(-\frac{x}{b}\right) \quad (7)$$

239 where \bar{A}_0 and \bar{B}_0 are the tidally averaged cross-sectional area and width at the estuary mouth; \bar{A}_r
 240 and \bar{B}_r are the asymptotic riverine cross-sectional area and width; and the parameters a and b
 241 represent the convergence lengths of the cross-sectional area and width. Here, by assuming that the
 242 cross-section approximates a rectangular shape, the tidally-averaged depth can then be calculated
 243 directly using $\bar{h} = \frac{\bar{A}}{\bar{B}}$.

244 For a predominantly tidal constituent with frequency ω , the tide-river hydrodynamics is mainly
 245 determined by four dimensionless parameters: the dimensionless tidal amplitude ξ (representing the
 246 boundary condition at the seaward side), the estuary shape number γ (representing the cross-

247 sectional area convergence), the friction number χ (representing the frictional dissipation) and the
 248 dimensionless river discharge φ (representing the influence of freshwater discharge imposed at the
 249 upstream boundary). Detailed definitions of these four parameters are shown in [Table S2](#).

250 The analytical solution for the main tide-river dynamics is obtained by solving a set of four
 251 dimensionless equations ([Cai et al., 2019](#)):

252 1) The damping/amplification equation,

$$253 \quad \delta = \mu^2(\gamma\theta - \chi\mu\lambda\Gamma)/(1 + \mu^2\beta) \quad (8)$$

254 where θ , β and Γ account for the effect of river discharge and are given by:

$$255 \quad \theta = 1 - (\sqrt{1 + \zeta} - 1)\varphi/(\mu\lambda) \quad (9)$$

$$256 \quad \beta = \theta - r_s\zeta\varphi/(\mu\lambda) \quad (10)$$

$$257 \quad \Gamma = \frac{1}{\pi}[p_1 - 2p_2\varphi + p_3\varphi^2(3 + \mu^2\lambda^2/\varphi^2)] \quad (11)$$

258 Note that Γ is a friction factor obtained by using the Chebyshev polynomial approach ([Dronkers,](#)
 259 [1964](#)), where the Chebyshev coefficients p_i ($i=0, 1, 2, 3$) are functions of the dimensionless river
 260 discharge φ through $\alpha = \arccos(-\varphi)$:

$$261 \quad p_0 = -\frac{7}{120}\sin(2\alpha) + \frac{1}{24}\sin(6\alpha) - \frac{1}{60}\sin(8\alpha) \quad (12)$$

$$262 \quad p_1 = \frac{7}{6}\sin(\alpha) - \frac{7}{30}\sin(3\alpha) - \frac{7}{30}\sin(5\alpha) + \frac{1}{10}\sin(7\alpha) \quad (13)$$

$$263 \quad p_2 = \pi - 2\alpha + \frac{1}{3}\sin(2\alpha) + \frac{19}{30}\sin(4\alpha) - \frac{1}{5}\sin(6\alpha) \quad (14)$$

$$264 \quad p_3 = \frac{4}{3}\sin(\alpha) - \frac{2}{3}\sin(3\alpha) + \frac{2}{15}\sin(5\alpha) \quad (15)$$

265 2) The phase lag equation

$$266 \quad \tan(\varepsilon) = \lambda/(\gamma - \delta) \quad (16)$$

267 3) The scaling equation

$$268 \quad \mu = \frac{\sin(\varepsilon)}{\lambda} = \frac{\cos(\varepsilon)}{\gamma - \delta} \quad (17)$$

269 4) The wave celerity equation

$$270 \quad \lambda^2 = 1 - \delta(\gamma - \delta) \quad (18)$$

271 The main dependent parameters are also described in [Table S2](#), including the
 272 amplification/damping number δ , which indicates the rate of increase ($\delta > 0$) or decrease ($\delta < 0$) of

273 the along-channel tidal wave amplitude; the velocity number μ , which indicates the ratio of the actual
274 velocity amplitude to the reference value in a frictionless prismatic channel; the celerity number λ ,
275 which indicates the ratio of the classical wave celerity (c_0) to the actual wave celerity (c); and ε ,
276 which indicates the phase lag between high water and high water slack or between low water and low
277 water slack.

278 To reproduce the tide-river dynamics for the entire channel correctly, the reach was sub-divided
279 into multiple segments to account for the longitudinal variations of the cross-sections. Thus, the tidal
280 amplitude at any distance Δx upstream of the seaward boundary was obtained by simple explicit
281 integration for a given tidal amplification/damping number δ and tidal amplitude η_0 at the seaward
282 boundary:

$$283 \eta_1 = \eta_0 + \frac{d\eta}{dx} \Delta x = \eta_0 + \frac{\eta_0 \omega \delta}{c_0} \Delta x \quad (19)$$

284 Based on the computed tidal amplitude and the geometric features of the next reach, the main tidal
285 dynamics, including the amplification/damping number, velocity number, celerity number and phase
286 lag, were obtained by solving the set of Eqs. (8), (16), (17), (18).

287

288 **3.2.3. Model Set-Up**

289 The analytical model for the Datong-Xuliujing Reach (DXR) was set-up using three groups of data
290 as follows: (1) geometric characteristics of the channel, including the cross-sectional area and channel
291 depth, were used to obtain the representative exponential functions (Eqs, 6 and 7); (2) model boundary
292 conditions, including the seaward tidal amplitude at Xuliujing and landward fluvial discharge at
293 Datong, were used to drive the model, and; (3) observed tidal amplitude and residual water levels for
294 the main stations along the DXR were employed for model calibration and verification.

295

296 **3.2.4. Calibration and verification of the analytical model in the Datong-Xuliujing Reach**

297 The analytical model for the DXR extends over a length of 512 km. The geometric characteristics
298 along the DXR for the pre- and post-TGD periods, respectively, were extracted from the DEM in
299 2002 and 2013, at intervals of 5 km. It is evident that both the cross-sectional area and stream width
300 are well represented by Eqs. (6) and (7), which converge exponentially toward a constant cross-
301 section landward in the river part with a statistically significant correlation ($p < 0.05$; Fig. S3 and Table
302 S3).

303 The analytical model was calibrated and verified against the observed tidal amplitude and residual
304 water level along the DXR on the basis of the monthly mean hydrological data (including tidal ranges
305 and water levels) in 2002 and 2013. The seaward tidal amplitude at Xuliujing, and the landward
306 fluvial discharge at Datong for these two time periods are displayed in the supplementary information
307 (Fig. S2, S4). As the Changjiang estuary is characterized by a semi-diurnal tide, the predominant M_2
308 tidal period (12.42 h) was used in the analytical model. The storage width ratio r_3 was set to a value
309 of 1 for simplification, indicating negligible influence of storage area on tidal dynamics (Cai et al.,
310 2013; Cai et al., 2019). Thus, the only calibration parameter is the Manning-Strickler friction
311 coefficient, K . In 2002, the calibrated value of K was $85 \text{ m}^{1/3}\text{s}^{-1}$ in the seaward region ($x=0-70 \text{ km}$),
312 decreasing to $50-85 \text{ m}^{1/3}\text{s}^{-1}$ over the transitional reach ($x=70-90 \text{ km}$), and then a further reduction to
313 $50 \text{ m}^{1/3}\text{s}^{-1}$ in the landward region ($x=90-512 \text{ km}$). In 2013, the calibrated value of K was $75 \text{ m}^{1/3}\text{s}^{-1}$ in
314 the seaward region ($x=0-70 \text{ km}$), decreasing to $43-75 \text{ m}^{1/3}\text{s}^{-1}$ over the transitional reach ($x=70-90 \text{ km}$),
315 and finally to $43 \text{ m}^{1/3}\text{s}^{-1}$ in the landward region ($x=90-512 \text{ km}$). Although the parameter K here is in
316 effect a ‘catch-all’ calibration parameter (reflecting not only the river bed friction, but also wind
317 forcing, the sinuosity of the channel, and so on; Cai et al., 2020), it is nevertheless noteworthy that
318 value of K decreases in the landward direction, which is consistent with observed spatial variations

319 in the bottom sediment grain size (the bed-material grain size being finer in the seaward areas). Note
320 also that the calibrated value of K decreases during the period 2002-2013, which is consistent with
321 the bed coarsening along the tidal reach.

322 The modeled tidal amplitude and residual water level were compared against observations at Wuhu
323 (WH), Maanshan (MAS), Nanjing (NJ), Zhenjiang (ZJ), Jiangyin (JY), and Tianshenggang (TSG)
324 through analysis of the Pearson product-moment correlation coefficient (Fig. 2). It can be seen that
325 for both the tidal amplitude and residual water level, all the correlation coefficients exceeded 0.99,
326 suggesting that the analytical model accurately reproduces the tide-river dynamics along the DXR.

327

328 **4. Scenario settings**

329 The calibrated analytical models before and after the operation of the TGD are used to run for a
330 range of simulation experiments, in which the boundary parameters driving the river-tide
331 hydrodynamics were varied and were calculated based on the observations during 1992-2013. Details
332 of the methods used to compute these key parameter values for the pre- and post-TGD periods of
333 interest are provided as follow:

334

335 ***4.1. Upstream boundary at Datong***

336 To simulate the river-tide hydrodynamics, the analytical model requires a time-invariant
337 reference discharge (specified here for the upstream limit of the DXR, at Datong). In this study we
338 conducted simulations using three different reference discharges, based on: (i) the effective discharge;
339 (ii) the 50th percentile (Q50) and; (iii) the 75th percentile (Q75) thresholds of the daily discharge
340 series. In all three cases these flow discharges were recalculated based on the daily water and sediment
341 discharge time-series at Datong over the periods 1998-2002 and 2009-2013, to represent the pre- and

342 post-TGD evolutionary stages, respectively.

343 Note that the effective discharge is defined as the stream flow that transports the largest amount of
344 sediment over geomorphic timescales (Wolman and Miller, 1960). The long-term geomorphic
345 effectiveness of a given discharge is, therefore, the product of the flow frequency ($f(Q)$) and the
346 suspended sediment transport rate ($S(Q)$) assigned to that flow (Q) (Doyle et al., 2005; Bunte et al.,
347 2014). The discharge that corresponds to the peak of the product function $E(Q)=f(Q)\times S(Q)$, is thus
348 defined as the effective discharge. The frequency ($f(Q)$) and the suspended sediment function ($S(Q)$)
349 that correspond to various discharge magnitudes were calculated using power functions as illustrated
350 in (Fig. 3 A-B, D-E). In this way, the effective discharges were estimated to be 47100 m³/s and 40100
351 m³/s, respectively, for the pre- and post-TGD periods (Fig. 3 C, F; Table 1). In addition, the Q50 and
352 Q75 during the pre- and post-TGD stages were computed to have values of 28000 m³/s and 41200
353 m³/s for the pre-TGD scenario; versus 23300 m³/s and 37800 m³/s for the post-TGD scenario (Table
354 1).

355

356 **4.2. Downstream boundary at Xuliujing**

357 The tidal amplitude at the downstream limit of the DXR was obtained using the hourly tide level
358 at Xuliujing station. Specifically, observations in 2002 and 2013 were selected to represent the pre-
359 and post-TGD scenario, respectively, because the tides in the Changjiang estuary are approximately
360 the same over each spring-neap tidal cycle (Zhang et al., 2016). During the simulation experiments
361 using the different flow discharge inputs (see 4.1) to represent the pre- and post-TGD scenarios, the
362 downstream boundary at Xuliujing was set as the synchronous monthly mean tidal amplitude over
363 the wet season. We focus on the tide-river hydrodynamics during the wet season as it is assumed that
364 the channel dynamics are shaped during higher flows, and because the channel bathymetric data used

365 to represent the channel morphology was collected at this time of year. The specified tidal amplitudes
366 were estimated as 1.04 m and 1.12 m for the pre- and post-TGD scenarios, respectively. An overview
367 of all the simulation experiments that were performed using the the analytical model are shown in
368 **Table 1**.

369

370 **5. Results**

371 *5.1. Erosion and accretion in the Datong-Xuliujing Reach during 1992-2002*

372 According to measurements made at Datong station, the Changjiang River annually transported
373 3.18×10^8 t of suspended sediment to the DXR during the period 1992-2002 (**Fig. S1**). Meanwhile,
374 during this same period, our bathymetric change analysis reveals that the DXR showed clear evidence
375 of accretion (**Fig. 4**), at a mean rate of 0.04 m/y (**Fig. 5A**). In total, the reach experienced net
376 deposition of 0.75×10^8 t/y, which is the equivalent of 23.6% of the annual load passing through
377 Datong. The deposition in this period resulted in a rising of the thalweg (**Fig. S5**) as well as a reduction
378 in the cross-section area of the channel (**Fig. S6**). Especially in the vicinity of Wuhu and Nanjing, the
379 mean elevation aggraded by 4.42 m and 5.73 m during 1992-2002, respectively (**Fig. S6A-B**).
380 Regarding the spatial pattern of bed accretion, there is a notable peak at the upstream limit of the
381 reach, some 50-80 km downstream from Datong. However, when this outlier is excluded, the pattern
382 of deposition with distance along the DXR conforms to a statistically significant ($p < 0.05$) Gaussian
383 distribution with distance along the channel, with the time-averaged rate of riverbed deposition a
384 maximum (~ 0.13 m/y) in the vicinity of Nanjing, which is some 230-280 km downstream from
385 Datong (**Fig. 5A**). The computed Gaussian fit has a standard deviation of 2.35 sub-reaches, revealing
386 that 68% of this zone of maximum deposition is located within a band between 200-320 km
387 downstream of Datong.

388

389 *5.2. Erosion and accretion in the Datong-Xuliujing Reach after 2003*

390 Following closure of the TGD in 2003, the suspended sediment load at Datong abruptly decreased
391 from 2.06×10^8 t in 2003 to 1.32×10^8 t in 2008, stabilizing at a value of around 1.30×10^8 t thereafter
392 (Fig. S1B). At the same time the channel also experienced riverbed scour throughout its length (Fig.
393 4), at an overall (along the full DXR) time-averaged erosion rate of 0.03 m/y during 2002-2008 (Fig.
394 5B), but with the mean bed elevation at Wuhu and Nanjing down-cutting by 5.24 m and 6.48 m,
395 respectively (Fig. S6A-B). Furthermore, during this erosional phase there is (unlike for the pre-TGD
396 period) there is no systematic spatial structure evident in the spatial pattern of morphological change
397 along the reach.

398 In the subsequent 5 years from 2008 to 2013, almost the entire DXR again experienced channel
399 aggradation, except in the reach downstream of Jiangyin, which underwent down-cutting (Fig. 4).
400 The average deposition rate of the DXR during this period was 0.07 m/y (Fig. 5C), when the river
401 bed elevations at Wuhu and Nanjing, respectively, increased by 5.81 m and 1.07 m (Fig. S6A-B).
402 Similar to the pre-TGD period (1992-2002), and neglecting the extremely low deposition zone,
403 located some 180-230 km downstream from Datong, as well as the zone of erosion in the distal part
404 of the reach, the spatial pattern of riverbed deposition during 2008-2013 again is seen to conform
405 well to a Gaussian distribution, but with the zone of maximum accretion now located close to Wuhu,
406 some 130-180 km downstream of Datong, indicating a landward shift of ~100 km compared with the
407 pre-TGD period (Fig. 5C). In this most recent time interval, the standard deviation around the
408 Gaussian distribution increases slightly to 2.42 sub-reaches, indicating that 68% of this zone of
409 observable deposition is located within a band between 120-250 km downstream of Datong.

410

411 5.3 River-tide dynamics along the Datong-Xuliujing Reach

412 The interactions between tide and river flow along the DXR were simulated using the analytical
413 model in terms of the residual (tide-averaged) water-level slope. The model results, corresponding to
414 an effective discharge of 47000 m³/s (pre-TGD) and 40000 m³/s (post-TGD) under wet season tidal
415 amplitudes of 1.04 m (pre-TGD) and 1.12 m (post-TGD) are illustrated in Fig. 6. It is shown that the
416 river-induced residual water level gradient in the pre-TGD period is considerably larger than in the
417 post-TGD phase, in part due to the larger fluvial discharge input in the pre-TGD period, and in part
418 to the increase in the tidal amplitude (Fig. S2, S4). The river-tide interaction induced residual water
419 level gradient is only evident in the lower reach, around 450-512 km from the Datong (Fig. 6). As the
420 tide propagates upstream, the tidal amplitudes attenuate, becoming very small at a location
421 approximately 300 km from Datong (Fig. 6). River forcing therefore dominates the dynamic process
422 of much of the tidally-influenced DXR in both scenarios.

423 The model results also show that there is a critical location along the DXR, at which the residual
424 water level slope is maximized. Specifically, in the pre-TGD stage, the longitudinal water level
425 gradient profile reaches its highest value of 2.21×10^{-5} at a location some 246 km downstream of
426 Datong (Fig. 6A). In post-TGD period, however, the location of the maximum residual water level
427 slope (at a much smaller value of 0.93×10^{-5}) occurs much further upstream, at a location 143 km
428 downstream of Datong (Fig. 6B). It is apparent that the locations of the model-simulated maximum
429 residual water level slopes coincide closely with the locations of the high deposition zones observed
430 during both the pre- and post-TGD periods (Fig. 6B, 6D).

431 Similar to the calculation of sub-reach averaged riverbed elevation change, the mean residual water
432 level slopes for the 20 sub-reaches are obtained for the depositional phases in Fig. 7A and 7C. It is
433 found that the longitudinal distribution of the residual water level slopes is statistical significantly

434 (p<0.05) correlated with the morphological changes for the river dominated reach, namely, the first
435 17 sub-reaches extending to around 435 km downstream of Datong (Fig. 7B, 7D), when the abnormal
436 morphological changes are excluded according with the former parts (see 5.1 and 5.2). This
437 phenomenon suggests the dominance of dynamics to the channel morphology. Besides, the total level
438 of residual water level slope for the post-TGD period is substantially below pre-TGD period due to a
439 damped high fluvial discharge and a relatively strong water level fluctuation, which in favor of
440 sediment settlement and thus contributes to the significant accretion during 2008-2013.

441

442 **6. Discussion**

443 *6.1. The effect of river morphology*

444 The geometric characteristics of a tidal reach to a large extent determine the propagation of the
445 tidal wave and hence the sediment transport dynamics (Zhou et al., 2018; Zhang et al., 2019). Before
446 the construction of TGD, there was a sufficient suspended sediment supply of 3.18×10^8 t/y from the
447 river (Fig. S1B), which generated overall deposition along the DXR, albeit with some localized areas
448 of erosion. Specifically, erosion was evident in the reach ~20-30 km downstream of Datong, where
449 the channel width decreases by over 35% from 3.07 km to 1.98 km (Fig. 1D). The sudden shrinkage
450 of the channel likely generates a different velocity distribution and as a result, riverbed erosion (Rouse,
451 1961). On the contrary, there was a significant deposition around 60-80 km downstream of Datong,
452 where the channel makes a dramatic U-turn, coupled with a sudden channel widening from 1.34 km
453 to 4.27 km, making the reach a naturally depositional environment (Fig. 1E). The channel
454 downstream of this high deposition zone shows a typical Gaussian profile in terms of the pattern of
455 morphological variation with respect to distance along the channel, due to the river-tide interaction,
456 which is further explained in Section 6.2.

457 Following the construction of the TGD, fluvial sediment supply decreased by 45.9% to 1.72 t/y
458 during 2003-2008, whereas the water discharge only decreased by around 10% (Fig. S1). However,
459 siltation still can be detected, particularly around the area where the channel width suddenly increases,
460 indicating the effects of sediment decline can be modulated by local channel planform variations.
461 However, the entire DXR shifted to a depositional state again during 2008-2013, when the suspended
462 sediment input to the DXR was stable at a value of around 1.30 t/y. Owing to channel resistance
463 (Calle et al., 2017), a stable armor layer can be formed in the surface of the river bed (Lai et al., 2017),
464 which prevents further channel degradation and contributes to morphological recovery. Consequently,
465 the morphological evolution again follows the Gaussian pattern with respect to distance along the
466 channel.

467

468 *6.2 Interaction between runoff and tidal impacts*

469 As a tide propagates into the tidal reach, river discharge attenuates the effects of the tidally-induced
470 water level oscillation, but in a way that is strongly related to the pattern of morphological evolution
471 along the estuary (Lamb et al., 2012; Leonardi et al., 2015). In this study, the relative contributions
472 of the tide-induced water level oscillation and river discharge along the DXR are further discussed
473 by considering various scenarios in the analytical model (Table 1). We focus on the mean-high
474 discharge inputs, as they are the main contributors for sediment transport in the Changjiang River
475 (Dai et al., 2016).

476 Two scenarios, corresponding to the normal (Q_{50}) and flood (Q_{75}) discharge conditions,
477 respectively, are examined for both the pre- and post-TGD stages (Fig. 8). As Fig. 8A indicates,
478 during the pre-TGD stage, the residual water level slope increased to a maximum value of 1.36×10^{-5}
479 at a location 180 km downstream of Datong, reducing thereafter in the normal flood condition (28000

480 m^3/s) following the deceleration of the river flow and the growing dominance of the water level
481 fluctuation induced by the tidal wave. The slope profile under flood conditions ($41200 \text{ m}^3/\text{s}$) reaches
482 a maximum gradient at a location 230 km downstream of Datong, indicating some 50 km of seaward
483 shifting and an increase in magnitude of the slope of 0.6×10^{-5} (Fig. 8B). The residual water level
484 slope behaves in a similar way in the post-TGD phase. When the river discharge input at Datong
485 increases by 38.4% between the flood ($23300 \text{ m}^3/\text{s}$) and high flood ($37800 \text{ m}^3/\text{s}$) scenarios, the
486 occurrence of the maximum residual water level slope migrates 47 km downstream, with the
487 magnitude increasing by 0.4×10^{-5} (Fig. 8C-D), indicating that the relative intensity of the river and
488 tidal forcing has the potential also to affect the location of the high deposition zone (because the
489 convergence of the slope to its maximum value would favor sediment deposition in that location). In
490 the high flow scenario, the effect of the tide is relatively weak, thus facilitating sediment deposition
491 further downstream, as well as in the high deposition zone. On the contrary, the tidal influence is
492 relatively strong in the low flow case, so that the location of the flow spreading and high deposition
493 zone exhibits a landward migration as the tide propagates further upstream (Sassi and Hoitink, 2013).

494

495 *6.3. Local mining and dredging*

496 The evolution of the DXR is also affected directly by human activities, notably, local mining and
497 dredging that directly force changes in the channel morphology (Zheng et al., 2018). The riverbed of
498 the DXR comprises mainly medium-fine sand (Fig. 9), a material which has beneficial characteristics
499 to many industries, but particularly as a construction material. Sand extraction was widespread in the
500 middle and lower Changjiang River in the 1990s, when the channel lost as much as 80 Mt of sediment
501 annually (Du et al., 2016). Commercial sand mining along the lower and middle Changjiang River
502 has declined since 2003 due to stronger regulation, with the total amount of sand extraction since then

503 being restricted to 14 Mt/yr (Chen et al., 2006). However, the actual quantity of sand mining has very
504 likely been considerably larger than the allowed amount due to illegal extraction. For instance, in a
505 small area 30 km upstream of Xuliujing, a total amount of 9.75 Mt of sand disappeared in half a year
506 from November 2011 to July 2012 (Liu et al., 2014), inducing severe erosion of the riverbed and
507 increased the channel capacity, destroying the Gaussian pattern of morphological variation noted
508 along the reach (Fig. 5C).

509 Moreover, intensive dredging operations are carried out along DXR for the purpose of improving
510 and maintaining the navigation channels, which is another human interference that further accelerates
511 channel down-cutting (Best, 2018). According to the plan of the 12.5 m Deep-Water Channel Project,
512 the reach downstream of Nanjing should be deepened to 12.5 m to improve navigable capacity (Fig.
513 1B). The implementation of this project caused an annual loss of $11.75 \times 10^6 \text{ m}^3$ of sediment from
514 downstream of Nanjing during the period 2012-2016 (Fig. S7). In comparison to 2003, before the
515 impoundment of the TGD, the averaged waterway depth and width of the DXR has respectively
516 increased by 1.48 m and 200 m by 2015 (Yang et al., 2019), resulting in severe riverbed erosion and
517 a landward moving of the deposition peak.

518

519 *6.4. Limitations and way forward*

520 The most restrictive assumption we have made in this study is to assume that the tidal wave can be
521 described by a combination of a steady residual term (river discharge) and a time-dependent harmonic
522 wave (tidal flow) in the analytical model. Thus the model can only deal with tide-river interactions
523 for a single predominant tidal constituent and fails to capture any tidal asymmetry introduced by
524 astronomical tides, overtides and compound tides. This is reasonable as the Changjiang estuary is
525 dominated by the M_2 semi-diurnal tidal constituent, followed (in order of importance) by the S_2 , K_1

526 and O1 constituents (Zhang et al., 2018). Located over 100 km away from the East China Sea, the
527 DXR is affected by the tide mainly in terms of water level fluctuation, rather than tidal asymmetry,
528 similar to the tidal reach of the alluvial Mississippi River (Wang and Xu, 2018). As Fig 6&8 indicates,
529 the residual water level gradient reaches its maximum value at a location between 88-246 km from
530 Datong, depending on the magnitude of the varying fluvial discharge. Over this range the tidal
531 amplitudes of S2, K1 and O1 are all very small and their fluctuations can be considered negligible
532 (Zhang et al., 2018). In addition, the analytical model is based on a tidally-averaged scale rather than
533 considering time varying processes caused by tidal action, it therefore fails to deduct the bed shear
534 stress profile and cannot be used to detect sediment transport along the channel directly. The focus
535 on such a time scale is acceptable in this study, given the long interval between the available
536 bathymetric surveys and the limited effects of tidal asymmetry on the tidal reach. Furthermore,
537 although the erosion and accretion patterns along the estuary are strongly related to the shape of the
538 residual water level profile (Lamb et al., 2012; Cai et al., 2019), further studies and investigations are
539 necessary to directly determine the mechanisms of net transport of sediment and thus directly link the
540 tide-river dynamics to the tidal reach morphology.

541

542 **7. Conclusions**

543 The tidal reach is a vital component of a river system, which directly links the upstream river basin
544 with the outer estuary. Influenced by both fluvial discharge and water level fluctuations due to tidal
545 action, the erosion/accretion states of a tidal reach have their own unique properties that contrast
546 sharply with those of the river and estuary. Using a case study of the DXR in the tidal reach of the
547 Changjiang River, this study has revealed the response of channel morphology to both river-tide
548 interactions and anthropogenic disturbances. The main conclusions are as follows:

549 1. The DXR experienced dramatic variations in morphology during 1992-2013, with a net
550 accretion of 0.04 m/y during 1992-2002, a net erosion of 0.03 m/y during 2002-2008, and again a net
551 accretion of 0.07 m/y during 2008-2013. The accretion status during 1992-2002 and 2008-2013 both
552 exhibit a distinct Gaussian variation with respect to distance along the channel.

553 2. Despite a rapid decrease in fluvial sediment supply due to the construction of the Three Gorges
554 Dam (TGD) in 2003, the DXR exhibits a clear maximum accretion zone during 1992-2002 and 2008-
555 2013, respectively, when the river-tide dynamics dominate the channel morphology along DXR. In
556 the natural scenario, during 1992-2002, the DXR exhibited a zone of maximum accretion that was
557 located around 230-280 km downstream of Datong; but this zone subsequently shifted ~100 km
558 landwards during 2008-2013.

559 3. The considerable decrease in high flow discharge during the wet season following the seasonal
560 operation of TGD for flood control is primarily responsible for the observed landward shift of the
561 maximum accretion zone within the DXR. However, the intrinsic characteristics of the local
562 geomorphologic configuration generate spatial nonuniformity in the overall pattern of
563 erosion/deposition, with localized sand mining and dredging projects in particular causing occasional
564 outliers of significant erosion/deposition.

565
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736 **Figure Captions**

737 **Fig. 1.** Overview of the Datong-Xuliujing Reach (DXR), showing: (A) the location of the Changjiang
738 River in China (the red box marks the location of panel B; TGD marks the location of the Three
739 Gorges Dam); (B) the DXR showing locations of sites referred to in the text; (C) thalweg profile of
740 the DXR as observed in 2013; (D-E) channel width variation in the selected reaches, with x-axis
741 indicating the distance from Datong (km) and the y-axis indicating the channel width. The remote-
742 sensing images of Fig. 1A is from USGS. The remote-sensing images of Fig. 1A is from USGS.

743 **Fig. 2.** Model calibration and verification through comparisons between modeled results and
744 observations for the periods before and after the operation of the Three Gorges Dam. Comparison of
745 (A) tidal amplitude and (B) residual water level against observations along the DXR in 2002;
746 comparison of (C) tidal amplitude and (D) residual water level against observations along the DXR
747 in 2013. The values of R indicate the correlation coefficient scores for the modelled versus observed
748 data.

749 **Fig. 3.** Derivation of effective discharge histogram (C, F) from sediment load rating curves (A, D)
750 and flow frequency (B, E) for the scenarios before and after the operation of the Three Gorges Dam.

751 **Fig. 4.** Bathymetric change maps indicating patterns of erosion and deposition along the DXR for
752 three epochs between 1992 and 2013

753 **Fig. 5.** Bathymetric changes observed along the Datong-Xuliujing Reach (DXR) during the period
754 1992-2013. For ease of description, the entire DXR is equally divided into 20 sub-reaches between
755 Datong and Xuliujing at intervals of 25.6 km: (A) annual rate of river bed elevation change during
756 the period 1992-2002; (B) annual rate of river bed elevation change during 2002-2008; and (C) annual
757 rate of river bed elevation change during 2008-2013.

758 **Fig. 6.** Longitudinal variation of the residual water level slope, and the contributions of tidal and
759 riverine forcing to the residual water level slope along the Datong-Xuliujing Reach for scenarios
760 representing: (A) the period prior to operation of the Three Gorges Dam and (B) the period after the
761 operation of the Three Gorges Dam. The vertical solid lines show the location of the maximum
762 residual water level gradient in each scenario.

763 **Fig. 7.** Residual water level gradient simulated along the Datong-Xuliujing Reach (DXR): (A) for
764 2002; (C) for 2013; and relationship between simulated residual water level gradient and the observed
765 yearly riverbed elevation change along the fluvial dominated reach during (B) 2002-1992 and (D)
766 2013-2008 (the blue line denotes the best fitting line). Red cycle indicates morphological change
767 outliers that are not considered in the correlation analysis.

768 **Fig. 8.** Longitudinal variation of the residual water level slope, and the contributions of tidal and
769 riverine forcing to the residual water level slope along the Datong-Xuliujing Reach for: (A-B) the
770 period prior to the operation of the Three Gorges Dam and (C-D) the period after the operation of the
771 Three Gorges Dam operation under (A, C) mean and (B, D) high fluvial discharge scenarios.

772 **Fig. 9.** Riverbed profile along the Datong-Xuliujing Reach.

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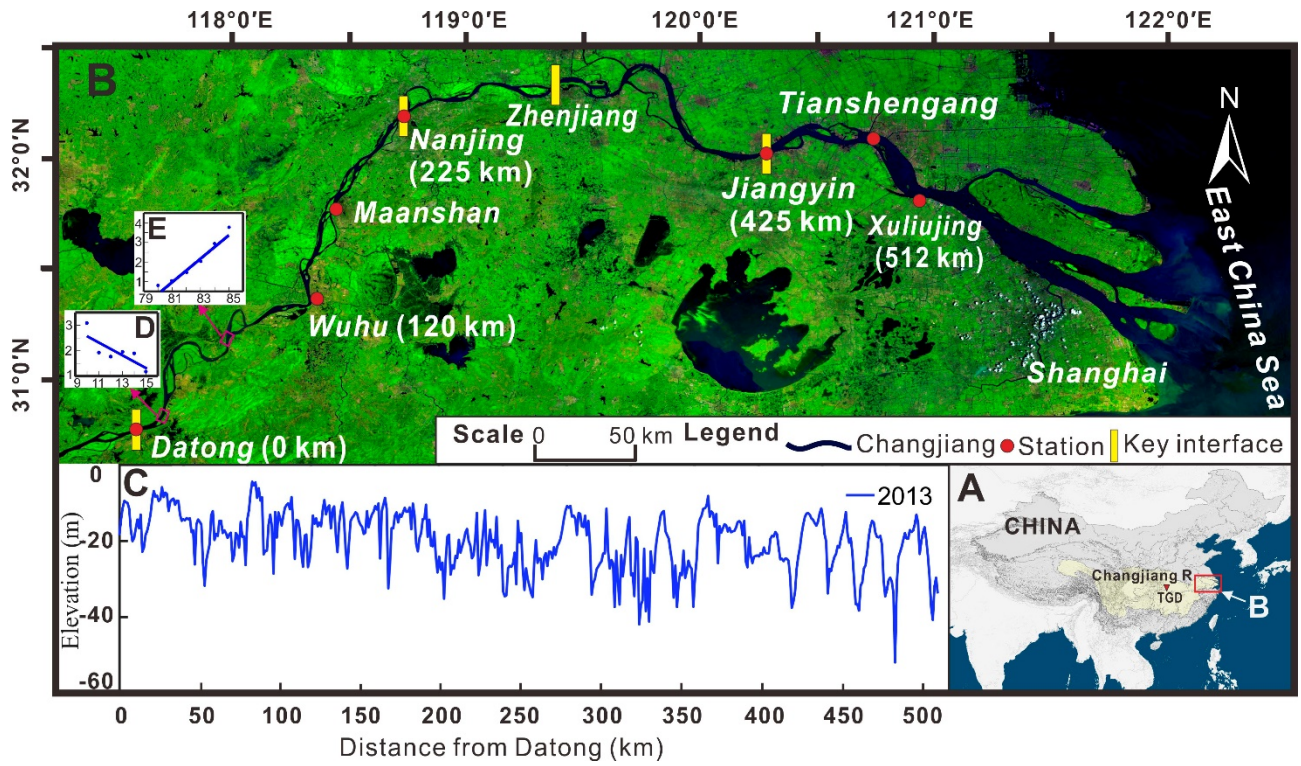
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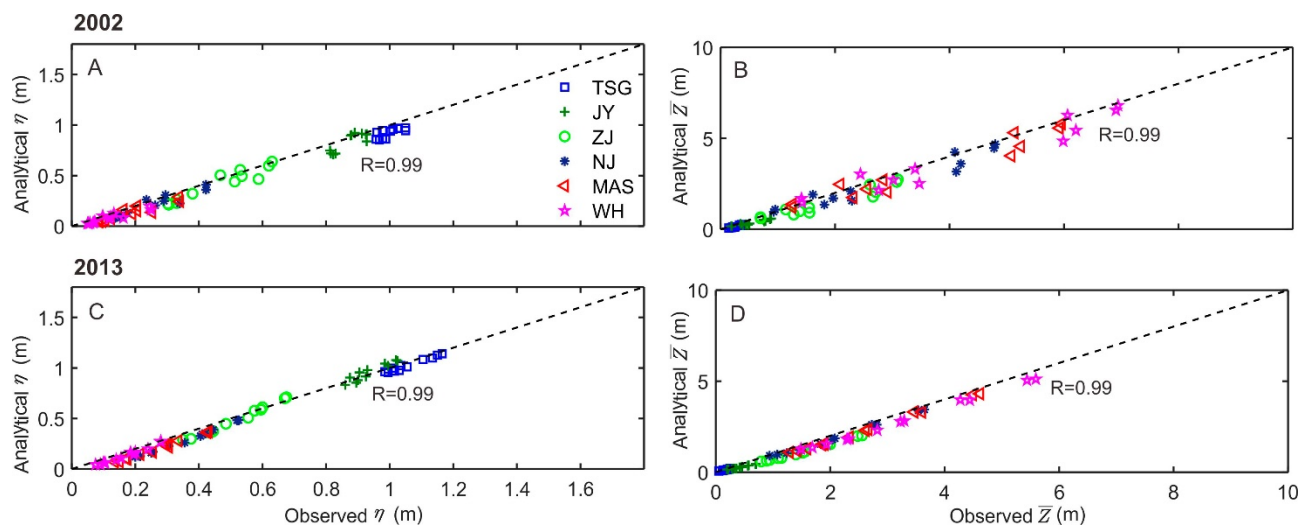
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781 **Fig. 1**



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783 **Fig. 2**



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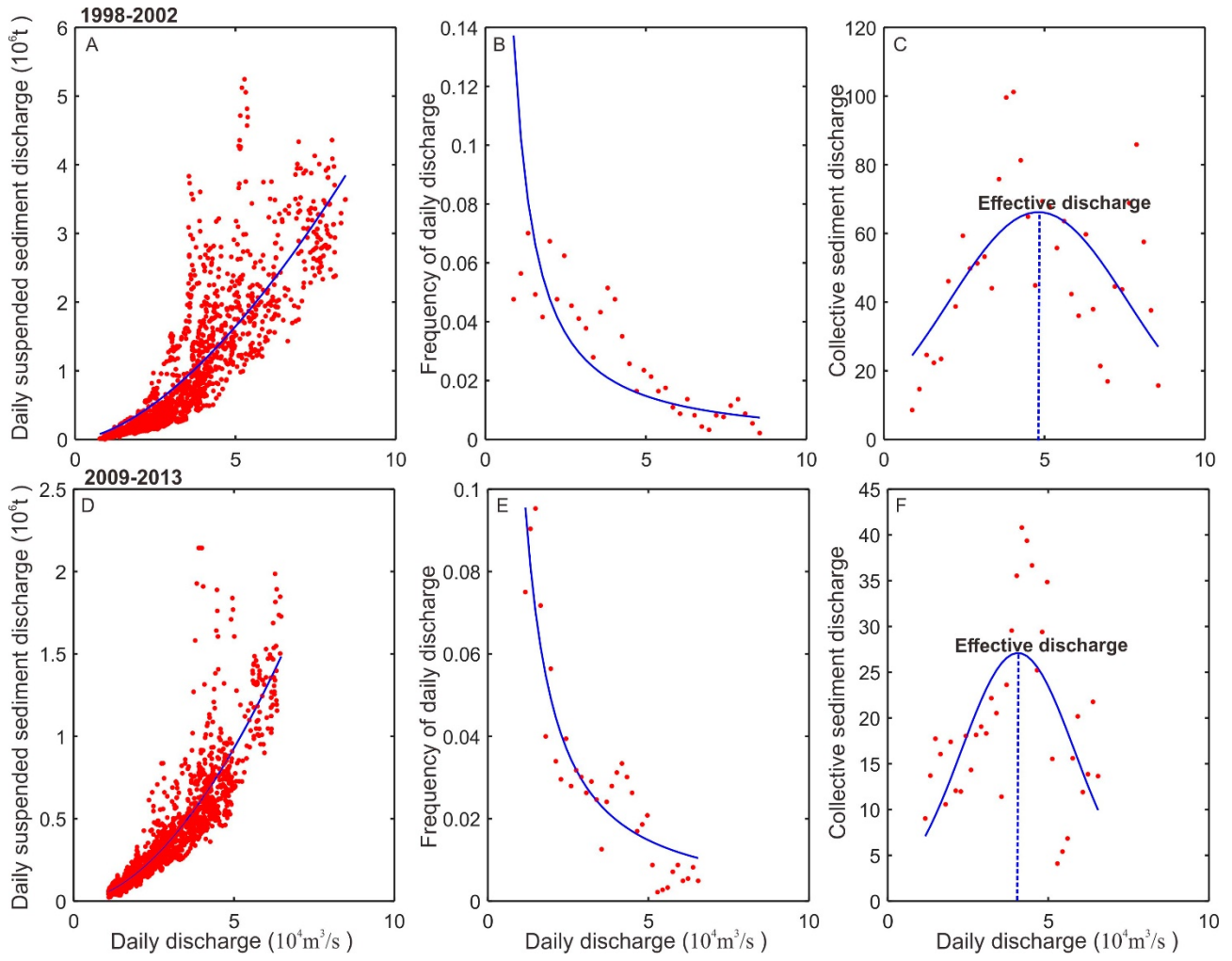
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790 **Fig. 3**



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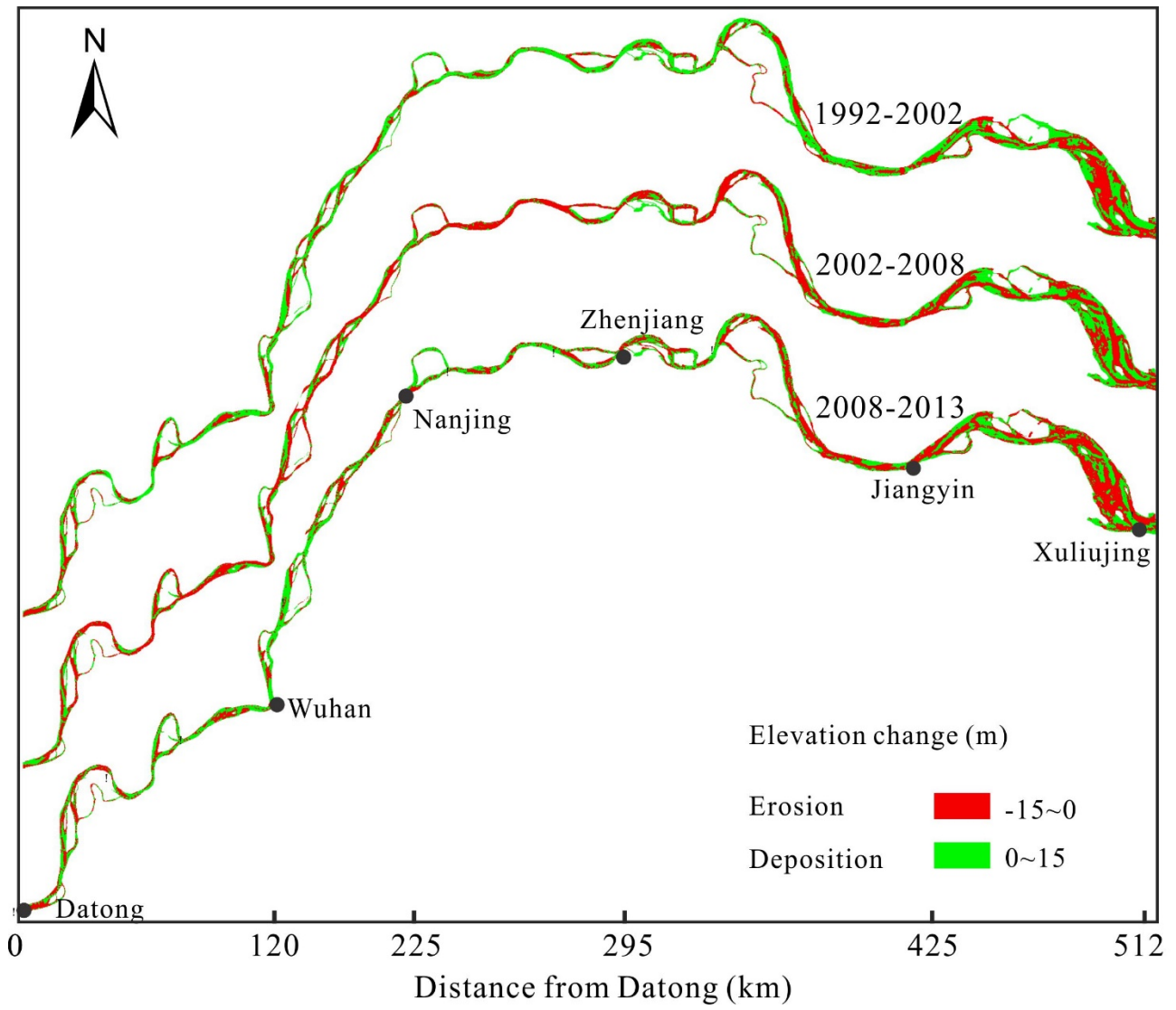
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802 **Fig. 4**



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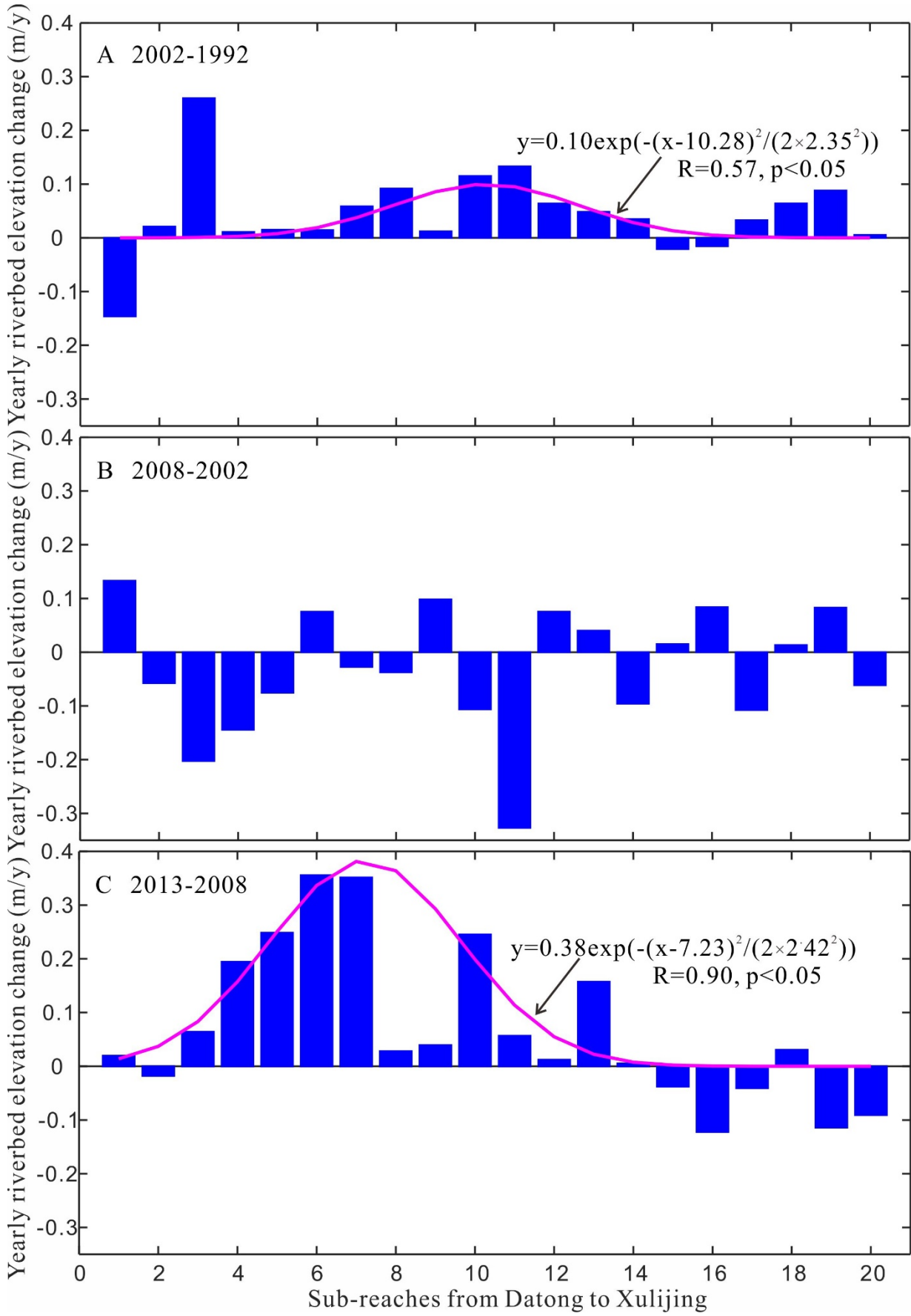
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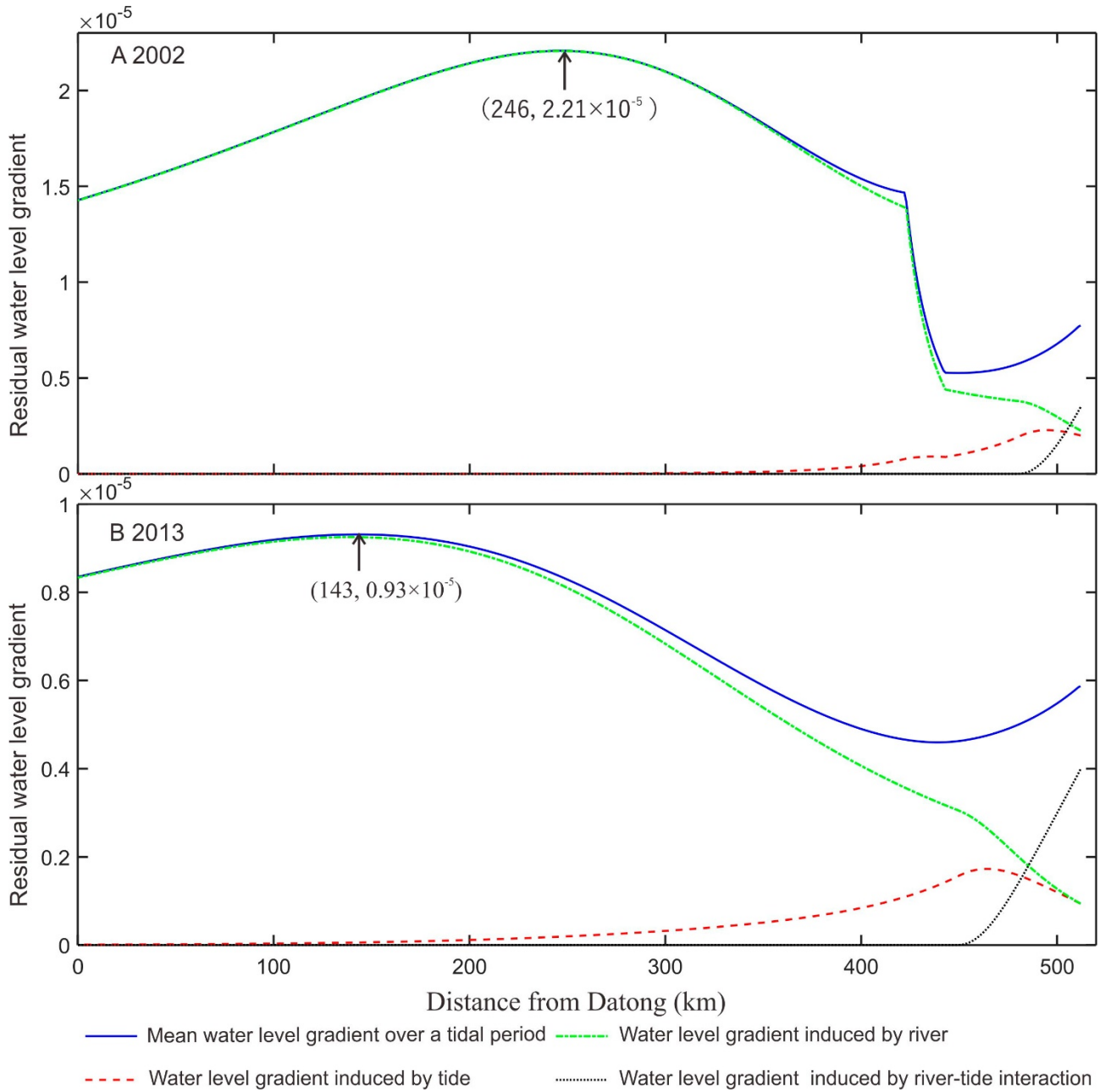
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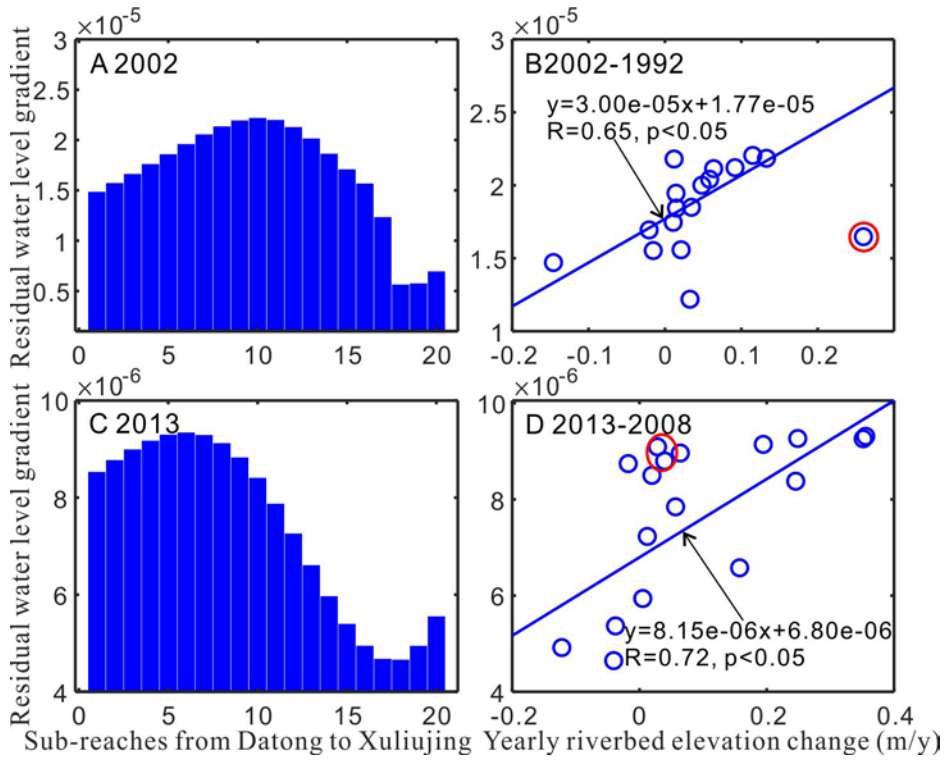
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825 **Fig. 7**



826 Sub-reaches from Datong to Xuliujing Yearly riverbed elevation change (m/y)

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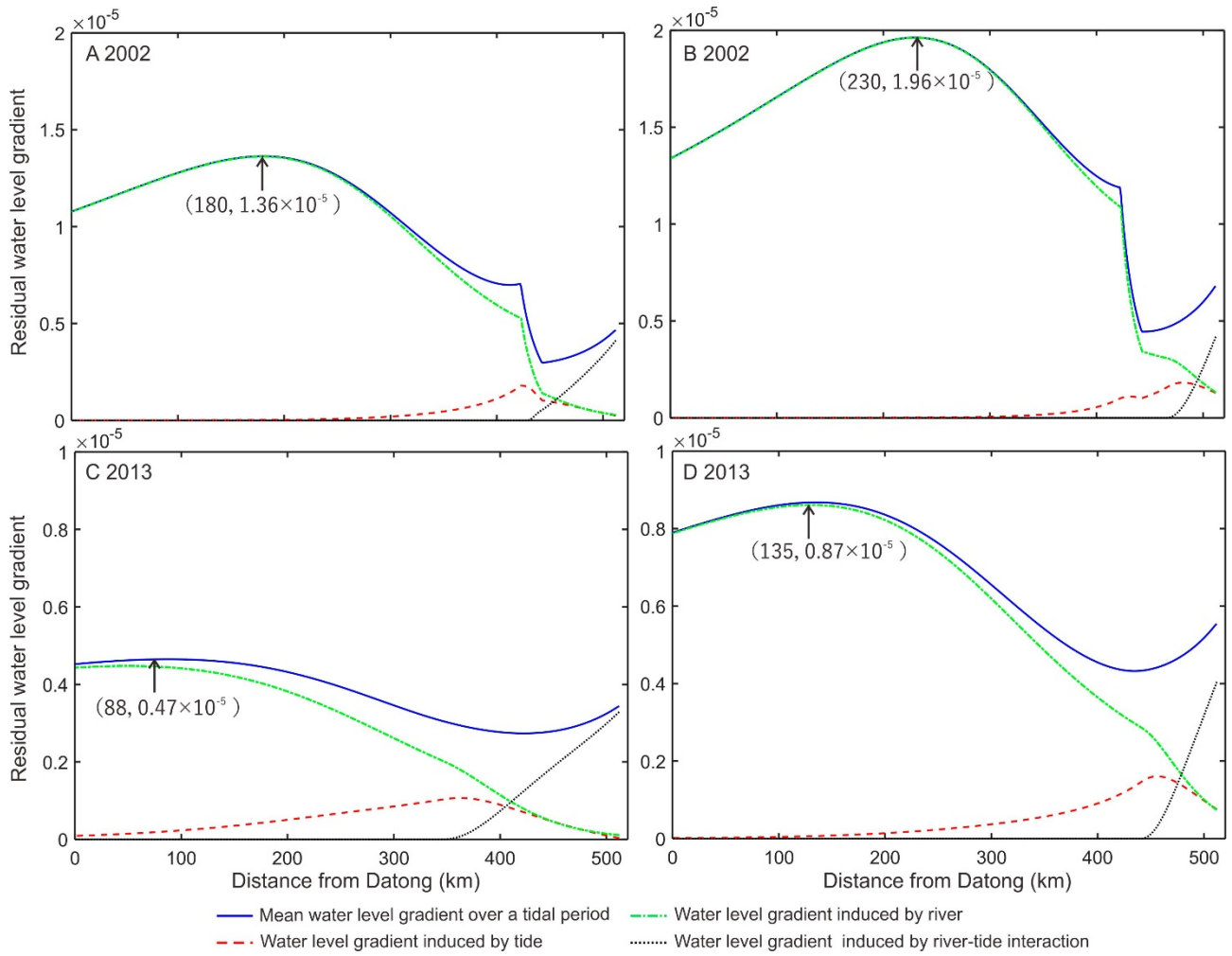
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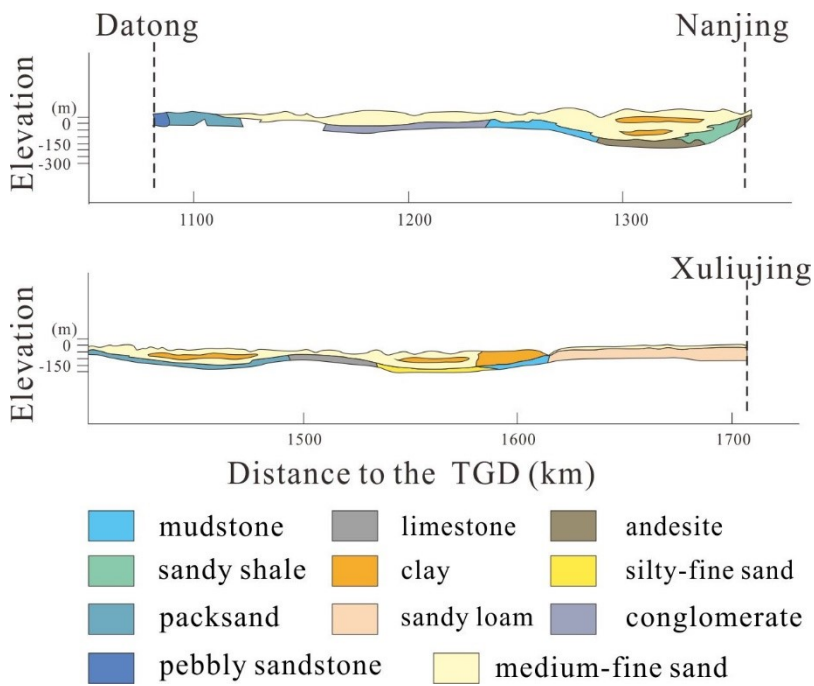
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840 **Fig. 8**



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842 **Fig. 9**



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845 **Table 1.** Overview of the simulation experiments in the analytical model

Scenario	Channel morphology	Upstream boundary (m ³ /s)	Downstream boundary (m)
		Effective discharge: 47100	
Pre-TGD	2002	Q50: 28000	1.04
		Q75: 41200	
		Effective discharge: 40100	
Post-TGD	2013	Q50: 23300	1.12
		Q75: 37800	

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866 **Landward shifts of the maximum accretion zone in the tidal reach of the Changjiang Estuary**
867 **following construction of the Three Gorges Dam**

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880 **Contents of this file**

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Table S1-A. Details about the bathymetric data of the Datong-Xuliujing Reach in 1992

Survey date	Map Title	Scale	Data sources
1992	Baimaosha waterway	1:60,000	CWB
1992	Qianjinkou-Xizhou	1:60,000	CWB
1992	Xizhou-Langshan	1:60,000	CWB
1992	Nantonggang	1:60,000	CWB
1992	Nanxing-Liuwei	1:40,000	CWB
1992	Zhangjiagang	1:40,000	CWB
1992	Dahegang-Rugaowei	1:40,000	CWB
1992	Rugaowei-Lu'anzhou	1:40,000	CWB
1992	Lu'anzhou-Guochuangang	1:40,000	CWB
1992	Guochuangang-Tiepigang	1:40,000	CWB
1992	Tiepigang-Heshangzhou	1:40,000	CWB
1992	Heshangzhou-zhengrunzhou	1:20,000	CWB
1992	Shiyezhou	1:40,000	CWB
1992	Shiyezhou-Longtan	1:40,000	CWB
1992	Longtan-Baguazhou	1:40,000	CWB
1992	Baguazhou waterway	1:20,000	CWB
1992	Jiangxinzhou	1:40,000	CWB
1992	Jiangxinzhou-Xinjizhou	1:40,000	CWB
1992	Xinjizhou-Xinhekou	1:40,000	CWB
1992	Xinhekou-Hongzhuang	1:40,000	CWB
1992	Hongzhuang-Wuhu	1:40,000	CWB
1992	Wuhu-Baodingwei	1:40,000	CWB
1992	Baodingwei-Tianranzhou	1:40,000	CWB
1992	Tianranzhou-Jinniudu	1:40,000	CWB
1992	Jinniudu-Chengdezhou	1:40,000	CWB
1992	Chengdezhou	1:40,000	CWB
1992	Henggang-Chongwenzhouwei	1:40,000	CWB

887 Note: CWB: Changjiang Waterway Bureau (CWB), Ministry of Transportation of China

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Table S1-B. Details about the bathymetric data of the Datong-Xuliujing Reach in 2002

Survey date	Map Title	Scale	Data sources
2002	Liuwenjing-Xuliujing	1:50,000	CWB
2002	Xincun-Beizhikou	1:50,000	CWB
2002	Qidonggang-Xincun	1:50,000	CWB
2002	Xuliujing -Tianshenggang	1:50,000	CWB
2002	Langshan- Shiyiwei	1:40,000	CWB
2002	Tianshenggang-Duanshan	1:40,000	CWB
2002	Duanshan-Ebizui	1:40,000	CWB
2002	Ebizui-Lianchengzhou	1:40,000	CWB
2002	Lianchengzhou-Shisiwei	1:40,000	CWB
2002	Shisiwei-Luochengzhou	1:40,000	CWB
2002	Luochengzhou-Jiaoshan	1:40,000	CWB
2002	Zhenjianggangqu	1:20,000	CWB
2002	Jiaoshan-Shierwei	1:40,000	CWB
2002	Shierwei-Qixiashan	1:40,000	CWB
2002	Qixiashan-Zhongshanmatou	1:40,000	CWB
2002	Nanjinggangqu	1:20,000	CWB
2002	Zhongshanmatou-Qiutingzhou	1:40,000	CWB
2002	Qiutingzhou-Hejiazhou	1:40,000	CWB
2002	Hejiazhou-Chenjiazhou	1:40,000	CWB
2002	Chenjiazhou-Dahewei	1:40,000	CWB
2002	Dahewei-Wanjiatan	1:40,000	CWB
2002	Wanjiatan-Zhangjiazhou	1:40,000	CWB
2002	Zhangjiazhou-Hejiachang	1:40,000	CWB
2002	Hejiachang-Nizhou	1:40,000	CWB
2002	Nizhou-Sanjiangkou	1:40,000	CWB

900 Note: CWB: Changjiang Waterway Bureau (CWB), Ministry of Transportation of China

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913 **Table S1-C.** Details about the bathymetric data of the Datong-Xuliujing Reach in 2008

Survey date	Map Title	Scale	Data sources
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2008	Liuwenjing-Baimaohe	1:40,000	CWB
2008	Xincun-Beizhikou	1:40,000	CWB
2008	Qidonggang-Xincun	1:40,000	CWB
2008	Lianxinggang-Qidonggang	1:40,000	CWB
2008	Baimaohe- Laohonggang	1:40,000	CWB
2008	Laohonggang-Tianshenggang	1:40,000	CWB
2008	Tianshenggang-Duanshan	1:40,000	CWB
2008	Duanshan-Ebizui	1:40,000	CWB
2008	Ebizui-Lianchengzhou	1:40,000	CWB
2008	Lianchengzhou-Shisiwei	1:40,000	CWB
2008	Shisiwei-Luochengzhou	1:40,000	CWB
2008	Luochengzhou-Jiaoshan	1:40,000	CWB
2008	Jiaoshan-Shierwei	1:40,000	CWB
2008	Shierwei-Qixiashan	1:40,000	CWB
2008	Qixiashan-Zhongshanmatou	1:40,000	CWB
2008	Nanjinggangqu	1:20,000	CWB
2008	Zhongshanmatou-Qiutingzhou	1:40,000	CWB
2008	Qiutingzhou-Hejiazhou	1:40,000	CWB
2008	Hejiazhou-Chenjiazhou	1:40,000	CWB
2008	Chenjiazhou-Dahewei	1:40,000	CWB
2008	Dahewei-Wanjiatan	1:40,000	CWB
2008	Wanjiatan-Zhangjiazhou	1:40,000	CWB
2008	Zhangjiazhou-Hejiachang	1:40,000	CWB
2008	Hejiachang-Nizhou	1:40,000	CWB
2008	Nizhou-Sanjiangkou	1:40,000	CWB

914 Note: CWB: Changjiang Waterway Bureau (CWB), Ministry of Transportation of China

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927 **Table S1-D.** Details about the bathymetric data of the Datong-Xuliujing Reach in 2013

Survey date	Map Title	Scale	Data sources
2013	Liuwenjing-Baimaohe	1:80,000	CWB
2013	Lianxinggang-Qidonggang	1:80,000	CWB

2013	Qidonggang-Miaogang	1:80,000	CWB
2013	Miaogang-Beizhikou	1:80,000	CWB
2013	Baimaohe- Laohonggang	1:80,000	CWB
2013	Laohonggang-Tianshenggang	1:80,000	CWB
2013	Tianshenggang-Duanshan	1:80,000	CWB
2013	Duanshan-Ebizui	1:80,000	CWB
2013	Ebizui-Lianchengzhou	1:80,000	CWB
2013	Lianchengzhou-Shisiwei	1:80,000	CWB
2013	Shisiwei-Luochengzhou	1:80,000	CWB
2013	Luochengzhou-Jiaoshan	1:80,000	CWB
2013	Jiaoshan-Shierwei	1:80,000	CWB
2013	Shierwei-Qixiashan	1:80,000	CWB
2013	Qixiashan-Zhongshanmatou	1:80,000	CWB
2013	Nanjingchangjiangdaqiao	1:40,000	CWB
2013	Zhongshanmatou-Qiutingzhou	1:80,000	CWB
2013	Qiutingzhou-Hejiazhou	1:80,000	CWB
2013	Hejiazhou-Chenjiazhou	1:80,000	CWB
2013	Chenjiazhou-Dahewei	1:80,000	CWB
2013	Dahewei-Wanjiatan	1:80,000	CWB
2013	Wanjiatan-Zhangjiazhou	1:80,000	CWB
2013	Zhangjiazhou-Hejiachang	1:80,000	CWB
2013	Hejiachang-Nizhou	1:80,000	CWB
2013	Nizhou-Sanjiangkou	1:80,000	CWB

928 Note: CWB: Changjiang Waterway Bureau (CWB), Ministry of Transportation of China

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Table S2. Definitions of the dimensionless parameters in the analytical model

Variables	Dependent variables
Dimensionless tidal amplitude	Amplification number
$\zeta = \eta/\bar{h}$	$\delta = c_0 d\eta/(\eta\omega dx)$
Estuary shape number	Velocity number

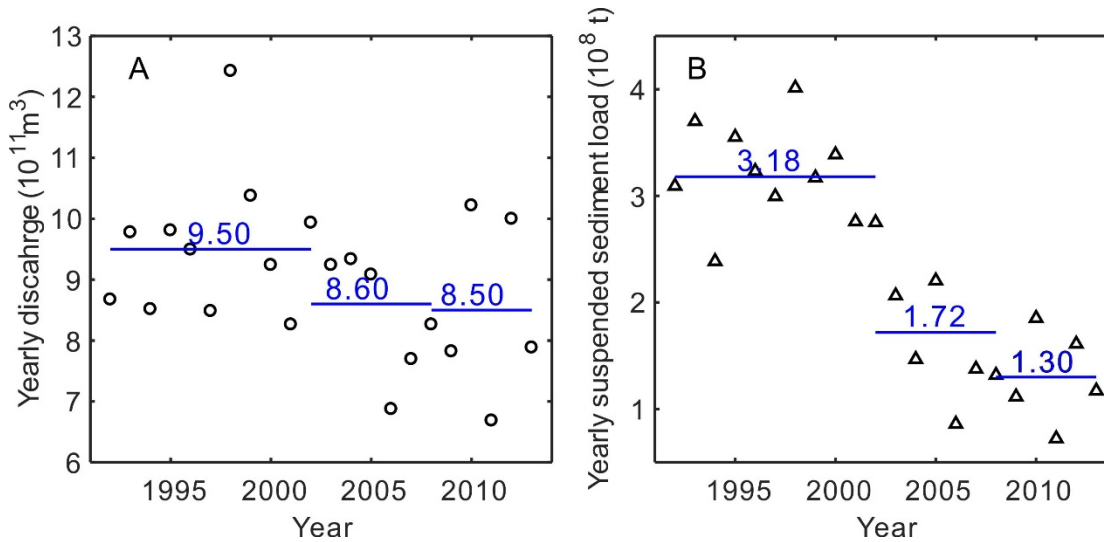
$\gamma = c_0(\bar{A} - \bar{A}_r)/(\omega\alpha\bar{A})$	$\mu = v/(r_s\zeta c_0) = v\bar{h}/(r_s\eta c_0)$
Friction number	Celerity number
$\chi = r_s g c_0 \zeta [1 - (4\zeta/3)^2]^{-1}/(\omega K^2 \bar{h}^{-4/3})$	$\lambda = c_0/c$
Dimensionless river discharge	Phase lag
$\phi = U_r/v$	$\varepsilon = \pi/2 - (\phi_Z - \phi_U)$

943 Note: η is the tidal amplitude; v is the velocity amplitude; $U_r = Q/\bar{A}$ is the river flow velocity; r_s is the storage
944 width ratio; $c_0 = \sqrt{g\bar{h}/r_s}$ is the classical wave celerity in a prismatic frictionless channel; ϕ_Z is the phase of
945 elevation; ϕ_U is the phase of current.

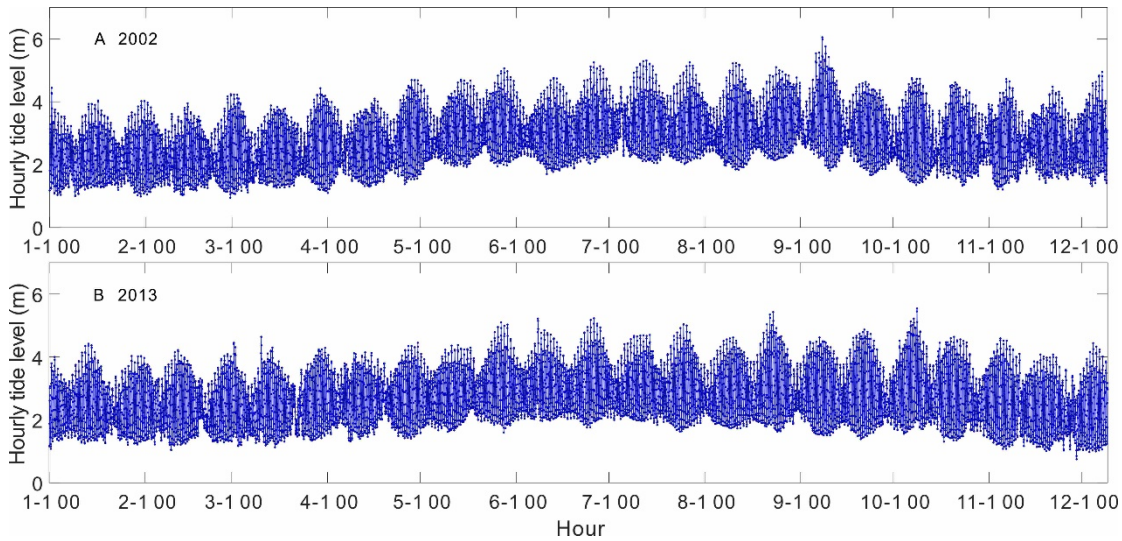
948 **Table S3.** Characteristics of geometric parameters in the Yangtze River estuary

Year	Characteristics	River	Mouth	Convergence length a or b (km)	R
2002	Cross-sectional area A (m ²)	17693	76138	126	0.97
	Width B (m)	2311	10110	62	0.91
2013	Cross-sectional area A (m ²)	22247	81255	121	0.96
	Width B (m)	2171	8778	74	0.89

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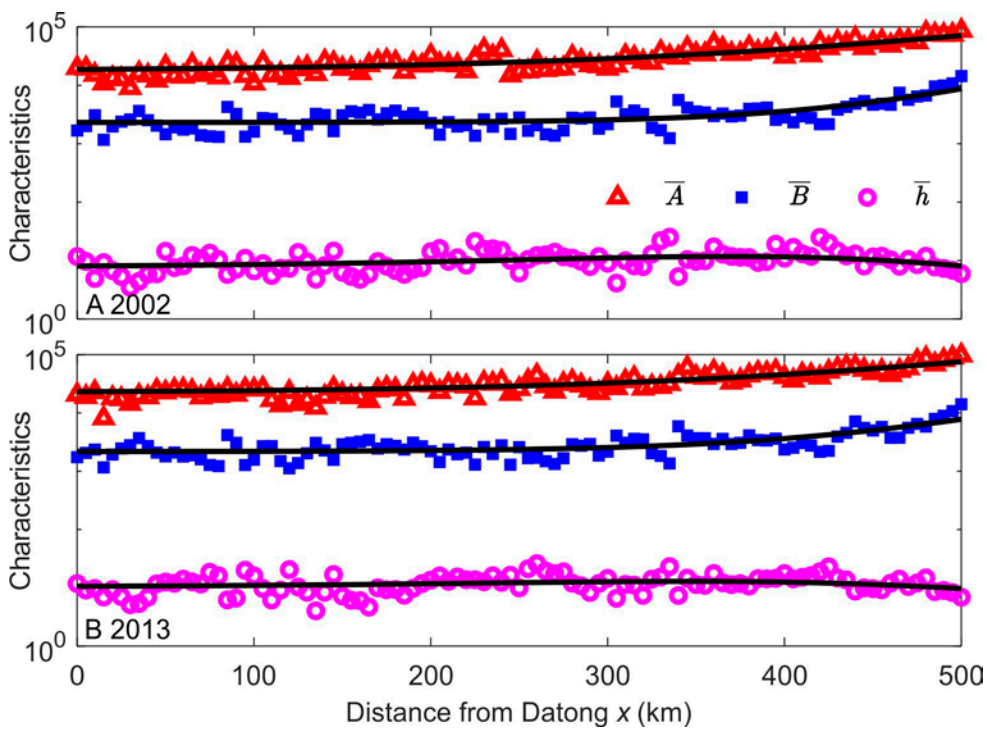


957 **Fig. S1.** (A) Yearly discharge (black cycle) and (B) Yearly suspended sediment discharge (black triangle) at Datong
958 station, with the blue solid line showing the mean value of each sub-phase.
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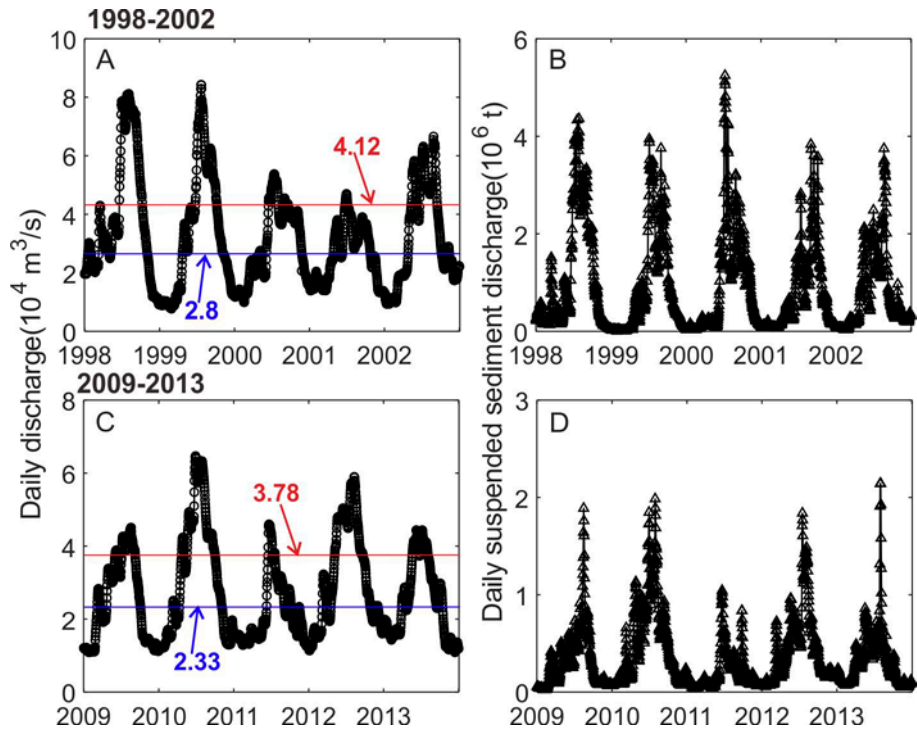
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Fig. S2. Hourly tide level in (A) 2002 and (B) 2013 at Xuliujing station.



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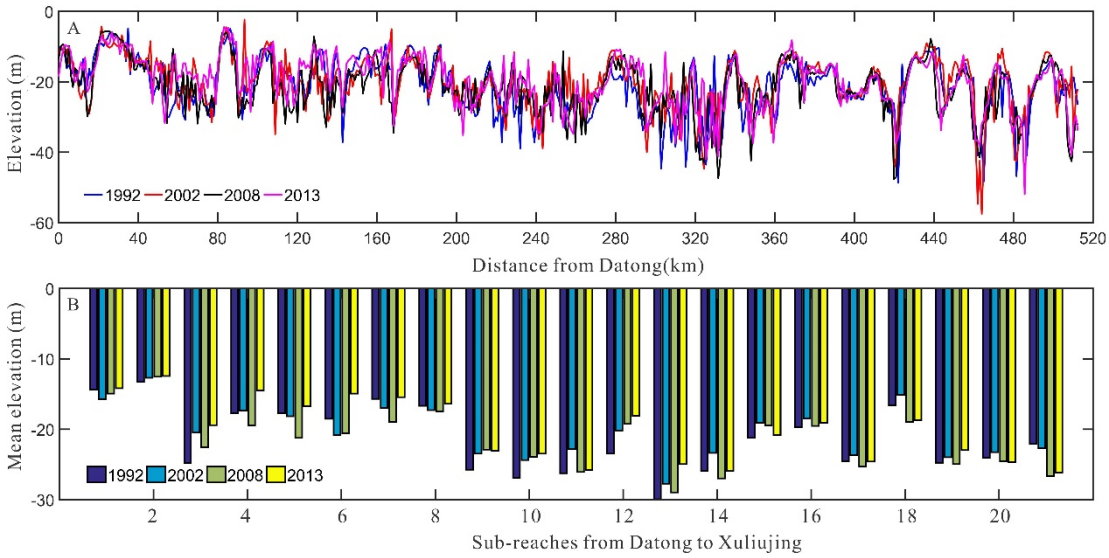
Fig. S3. Longitudinal variation of the main geometric characteristics (cross-sectional area, width and depth) along the Datong-Xuliujing Reach (from Xuliujing to Datong) at (A) 2002 and (B) 2013. The thick black lines represent the best-fitting curves.



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971 **Fig. S4.** Daily discharge (black cycle) and daily suspended sediment discharge (black triangle) during 1998-2002
 972 (A-B) and 2009-2013 (C-D) at Datong, with the red line indicating the value of Q75 while the blue line indicating
 973 the value of Q50.

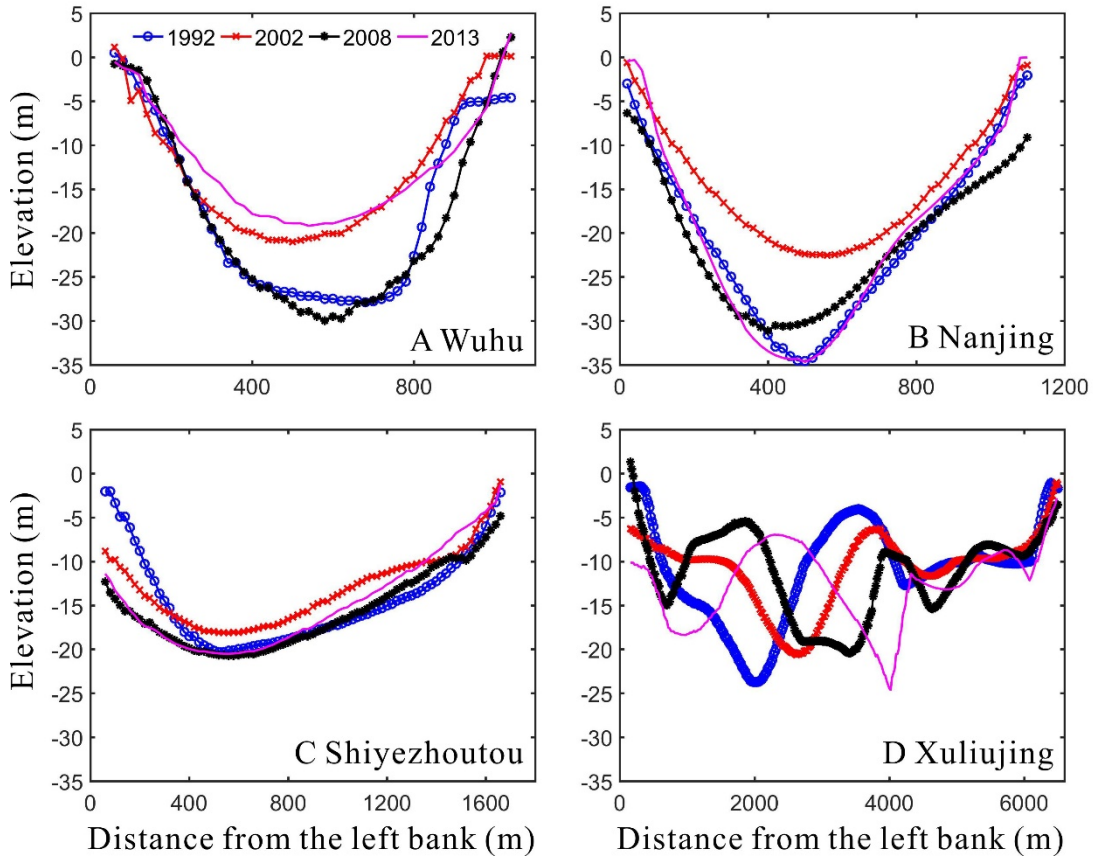
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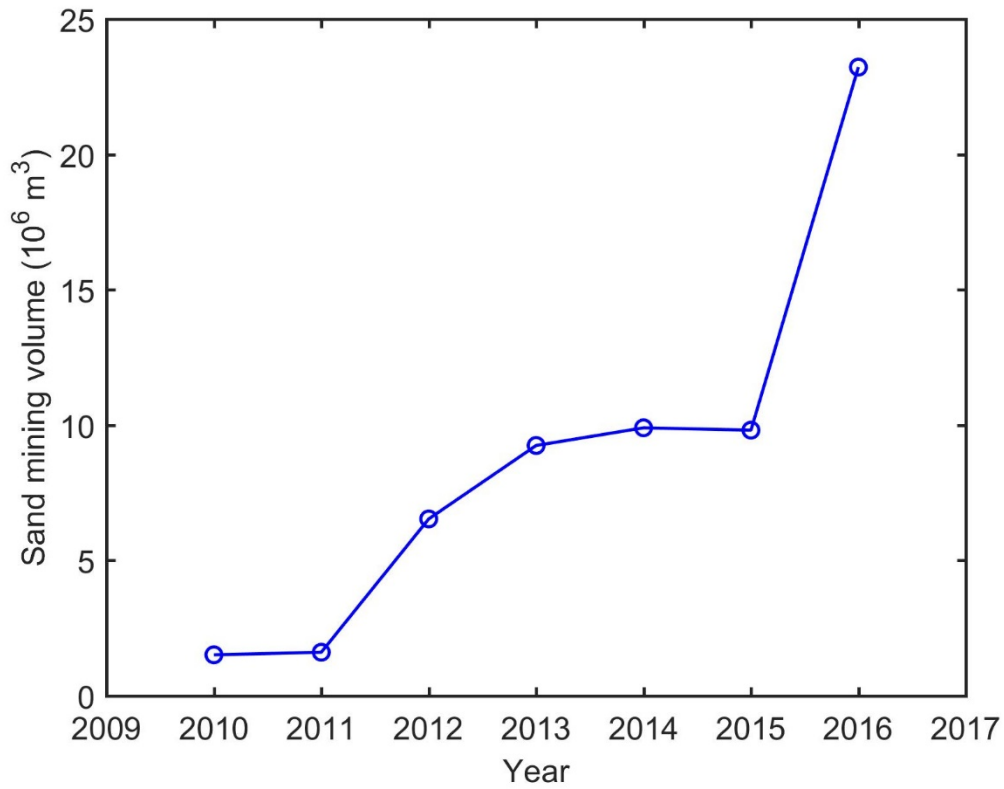
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976 **Fig. S5.** Thalweg profiles showing the elevation variation along the Datong-Xuliujing Reach, with (A) Thalweg
 977 profile along the DXR of different years and (B) the mean elevation of each sub-reaches of different years (the entire
 978 DXR is divided into 21 sub-reaches with an interval of 25 km for better showing the morphological variations).

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 981 **Fig. S6.** Cross sections showing the elevation changes along the Datong-Xuliujing Reach from 1992 to 2013: (A)
 982 Wuhu; (B) Nanjing; (C) Shiyezhoutou and (D) Xuliujing.



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 984 **Fig. S7.** Yearly sand mining volume for waterway dredging downstream of Nanjing
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